applied sciences

Review

New trends towards enhanced structural efficiency and aesthetic potential in tall buildings: The case of diagrids

Domenico Scaramozzino [1], Giuseppe Lacidogna [1,*] and Alberto Carpinteri [1]

[1] Department of Structural, Geotechnical and Building Engineering, Politecnico di Torino, Corso Duca degli Abruzzi 24, 10129, Torino, Italy; domenico.scaramozzino@polito.it, giuseppe.lacidogna@polito.it, alberto.carpinteri@polito.it

* Correspondence: giuseppe.lacidogna@polito.it; Tel.: +39-011-090-4871 (G.L.)

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Abstract: Due to the increasing number of people and activities within the cities, tall buildings are exploited worldwide to address the need for new living and commercial spaces, while limiting the amount of used land. In the last decades, the design of tall buildings has experienced a remarkable improvement thanks to the development of new computational tools and technological solutions. This has led to the realization of innovative structural systems, like diagrids, which allow to reach high structural performances and remarkable architectural effects. In this paper, a thorough and updated review of diagrid structural systems is provided. Simplified methodologies for the preliminary design and structural analysis are reported. Special attention is also paid to the optimization of the structural response based on the geometrical pattern. A discussion on the effect of local deformability, stability and shear-lag phenomenon is carried out. Results from nonlinear and dynamic analyses for the seismic assessment of diagrid systems are reported, and attention is also paid to the recent research on diagrid nodes. Eventually, an overview of twisted, tapered, tilted and freeform diagrid towers is carried out, with a final mention to hexagrids, another recent evolution of tubular systems for tall buildings.

Keywords: diagrid; preliminary design; structural analysis; stiffness-based methodology; optimization; hexagrid.

1. Introduction

The evolution of tall buildings has experienced remarkable developments in the last century. The first buildings reaching few tens of stories were firstly built in the United States in the late nineteenth century, mostly in the cities of New York and Chicago. At the beginning of the twentieth century, a race for the realization of the tallest skyscrapers took place, which led to the completion of the 102-story tall Empire State Building in 1931 (Figure 1a). Although at that time the height of those buildings was worthy of note, their realization was not achieved by means of significant technological innovations. They usually employed the same steel frames which were adopted for shorter buildings, leading to excessive material usage and quite over-designed solutions [1]. Bracings were employed to withstand lateral loads arising from wind pressures and earthquake actions. It was already recognized that lateral actions usually govern the design solutions in tall buildings. In fact, as the building becomes taller, the lateral drifts turn out to be more critical and there is greater demand of suitable structural systems to carry lateral forces. This leads to the dramatic increase of material consumption with the increase in the number of stories, which is usually referred to as the "premium for height" [1,2].

Due to aesthetic and constructability considerations, the bracings were usually embedded within the interior core of the building. Although their shear resistance based on the axial deformation of the diagonals was beneficial to resist the lateral actions, compared to the mechanism of the conventional moment resisting frames, their placement within the interior of the building prevented their effective employment to withstand the overturning moment. Therefore, new solutions exploiting bracings on the external perimeter of the building were developed. One of the first examples was the 100-story tall John Hancock Center built in Chicago in 1970 (Figure 1b). The John Hancock is an example of braced tube, where the mega-diagonals spanning over several stories are effective to resist the shear and bending moment deriving from lateral actions. The braced tube was a variation of the typical framed tube, where closely spaced perimeter columns were in charge of providing the necessary lateral stiffness. The adoption of mega-diagonals on the external surface offered higher lateral stiffness, while reducing some detrimental phenomena of the framed tube such as the shear-lag effect. With this new solution, higher number of stories and an overall enhanced structural performance could be achieved, leading also to important advantages from a material consumption perspective.

(**a**) (**b**)

Figure 1. Different structural systems for twentieth century tall buildings: (**a**) Moment resisting frame: Empire State Building (New York, USA); (**b**) Braced tube: John Hancock Center (Chicago, USA). Pictures taken by D. Scaramozzino.

Based on the structural behavior of the braced tube, where vertical columns and external bracings were designed to carry gravity and lateral loads respectively, it was realized that the external mega-diagonals were able to resist vertical and horizontal loads simultaneously, without the need of conventional vertical columns. This led to the realization of the diagrid ("diagonal" + "grid") structural system.

The idea of removing vertical columns and considering only inclined diagonals was not new. As a matter of fact, the first diagrid structure was realized before the construction of the John Hancock braced tube, in the 1920s by the Russian architect Vladimir Shukhov, for the realization of a broadcasting tower in Moscow [3]. The external pattern, made up of a triangular tessellation, allowed to reduce the wind load while reaching a stable stiff configuration. The first application of diagrid system in building design occurred in the 1960s, with the completion of the 13-story tall IBM Building (Pittsburgh, USA). The steel diagrid exoskeleton was integrated with the glazing system and assisted in the overall stability of the building [3].

However, it was not until the early twenty-first century that diagrid systems started to be thoroughly applied for the design and construction of tall buildings. The first examples are the Hearst Tower in New York (Figure 2a) and the 30 St. Mary Axe (also known as Swiss Re Tower or The Gherkin) in London (Figure 2b), both by Sir Norman Foster. These buildings allowed to reach 180 meters and provided the first references for the suitability of diagrid systems in tall building design. Thanks to the stiff diagrid façades which create a pleasant diamond-like pattern, the Hearst Tower was realized using 20% less steel than an equivalent conventional moment frame structure

[4]. The aerodynamic form of the Swiss Re Tower, obtained through an external free-form diagrid envelope, allowed to reduce the wind action on the building and led to column-free flexible internal spaces [5]. These two examples already showed the valuable features of diagrids for tall buildings: enhanced structural performance, saving of material consumption compared to traditional solutions, and significant aesthetic potential.

Many diagrid structures were realized worldwide in the following years, where various forms and shapes were adopted for the external diagrid façades. Among others, examples worthy of notes are the Guangzhou Financial Center, the CCTV Tower and the Poly International Plaza in China, the Tornado Tower (Figure 2c) in Qatar, the Capital Gate in Arab Emirates, and the Bow Tower in Canada [3]. Nowadays, most of the built diagrid structures are made of steel, mostly due to the easier and faster construction, simpler joints and less expensive formworks [3]. However, concrete and composite diagrids are also experiencing an increasing popularity, as they allow to realize even more complex-shaped diagrid patterns, e.g. the O-14 Building in Dubai [6].

(**a**) (**b**) (**c**)

Figure 2. Examples of diagrid systems in tall buildings: (**a**) Hearst Tower (New York, USA), picture taken by D. Scaramozzino; (**b**) Swiss Re Tower (London, UK), from https://larryspeck.com/; (**c**) Tornado Tower (Doha, Qatar), from http://www.asergeev.com/.

The significant use of diagrid systems in recent tall buildings was mainly due to the following reasons: (1) high lateral stiffness (thus low lateral deformability), which allowed to reach the lateral deflection limit target by using lower amount of structural material compared to other conventional systems; (2) architectural flexibility, allowing a more rational use of the interior space with fewer columns; (3) modularity, which led to the realization of complex-shaped structures of various forms. These three points arise from the successful use of the triangular module coupled with the inherent structural performance of the tubular structure [7,8].

The triangular element, which is made up of two inclined diagonals and a ring beam, is the basic component of the diagrid façade. The diagonals carry the vertical and lateral loads mostly by axial forces (compression or tension). For this reason, they are usually considered to be pinned at the panel nodes, as reflected in Figure 3a. Since the inclined diagonals often extend over multiple stories, the external floor beams of intermediate stories are often supported by the diagonals and consequently induce slight shear and bending stresses on them. However, in preliminary design stages, these are usually neglected when compared to the high axial stresses arising from the vertical and lateral loads on the building. Note that in Figure 3a, a 3-story module is reported as the height of diagrid module has the same height of the triangular element. In other research works, as will be shown in further figures in the remaining of the paper, the diagrid module is defined in a way that it covers two triangular elements.

In Figure 3b, the three-dimensional view of the tubular diagrid structure is shown, as reported in [9]. Usually, the tube-in-tube configuration is found in real diagrid buildings, where an internal (concrete or steel braced) core is coupled to the external diagrid tube. In preliminary design stages, the diagrid is usually designed to carry the lateral actions alone, while the internal core is designed only for gravity loads. However, further details about the diagrid-core interaction will be shown in the remaining of the paper.

Hence, it is the combination of the axial resisting mechanism of the triangular element, characterized by modularity and arrangement flexibility, coupled to the structural efficiency of the tubular configuration that ultimately led to the success of the diagrids in recent days.

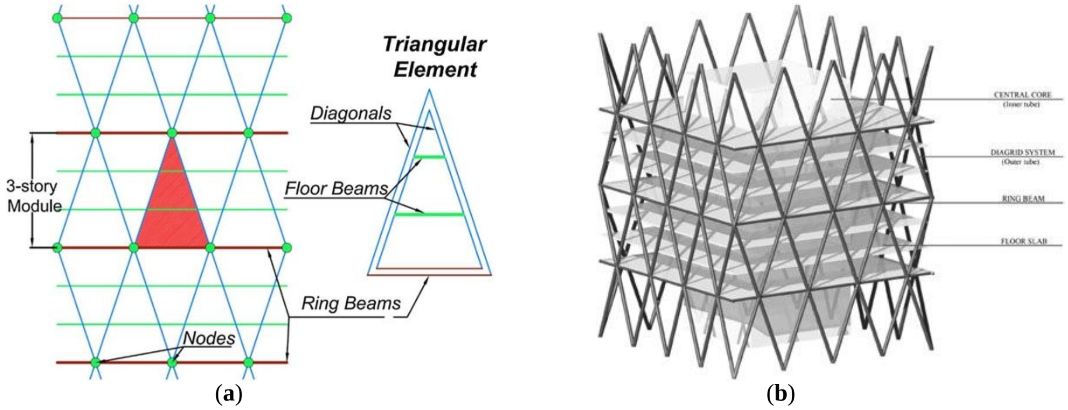


Figure 3. Fundamental diagrid geometrical features: (**a**) diagrid module and basic triangular element, used with permission from Asadi and Adeli [8]; (**b**) diagrid tubular configuration, used with permission from Angelucci and Mollaioli [9].

In this paper a thorough and up-to-date survey of the research studies on diagrid systems is reported. In particular, in Section 2 the fundamental stiffness-based approaches for the preliminary design is described, as firstly proposed by Moon et al. [10] and further developed in the following years. Moreover, strength-based design methodologies are also discussed and their implication on the preliminary design is analyzed. Section 3 describes the various methodologies available today, besides the typical Finite Element Method (FEM), for the structural analysis of diagrid structures in preliminary design stages, e.g. hand-based calculations, modular and matrix-based methods, etc. In Section 4, the subject related to the optimization of the diagrid performance based on its geometrical features is also addressed. This problem has been thoroughly tackled by various researchers in the last decade with different methodologies and has led to significant results. Section 5 describes the problem of local structural issues in the design of diagrid tall buildings, e.g. excessive inter-story drifts and stability of interior columns. The mathematical formulation to identify these problems is reported, as well as the solutions which have been suggested to tackle them, such as the insertion of secondary bracing systems (SBSs) as firstly proposed by Montuori et al. [11]. In Section 6, the shear-lag phenomenon in diagrid tubes is discussed and its influence depending on the diagrid geometrical parameters is analyzed. Section 7 also discusses the research studies which have dealt with the nonlinear behavior of diagrid tubes, in order to assess their seismic and robustness performance. In Section 8, the recent research on diagrid nodes, which represent a crucial component for the correct behavior of the diagrid, is also reported. Section 9 provides comments about the new trends regarding unconventional diagrids, which are applied to the realization of twisted, tilted, tapered and freeform buildings. A further evolution of the grid tubular structure, which has experienced a significant growth in recent tall building design, is finally presented in Section 10.

2. Simplified approaches for the preliminary design of diagrid tubes

The first simplified stiffness-based approach for the preliminary design of diagrid systems has been proposed by Moon et al. in 2007 [10]. It is based on the evaluation of the shear and bending stiffness of the diagrid modules, aimed at limiting the lateral deflection of the structure. The building is treated as a vertical cantilever beam, fixed on the ground and subjected to lateral loads. Accordingly, the building undergoes horizontal displacements, that depend on the stiffness of the diagrid tubular structure. For sake of the preliminary design, the contribution of the internal cores to the lateral stiffness of the building is neglected, as they are only designed to carry gravity loads.

The elementary diagrid module is depicted in Figure 4. The diagrid module covers an height h with two triangular elements. The diagonals have a length L_d and their inclination with respect to the horizontal plane is θ. Depending on the loading direction, each façade can act either as a web or a flange. V_i and M_i are the shear force and bending moment acting on the level of the i[th] module. These are carried by the web and flange diagonals, respectively. Diagonals are assumed to be pinned at their end, thus carrying only axial force, and remain in the linear elastic regime. In this way, the cross-sectional areas of the web and flange members are the only factors to obtain in order to accomplish the preliminary design.

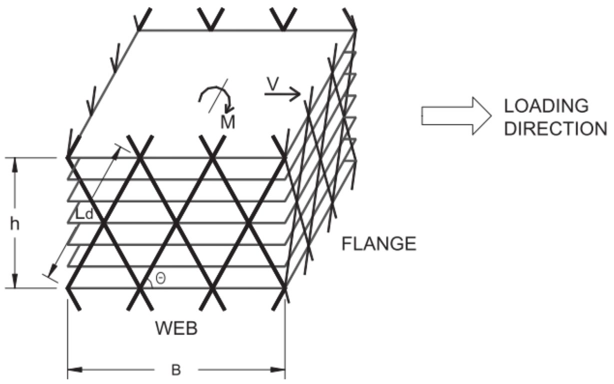


Figure 4. Scheme of the elementary diagrid module for the definition of the stiffness-based approach for the diagrid preliminary design. Used with permission from Moon et al. [10].

The shear stiffness $K_{T,i}$ and bending stiffness $K_{B,i}$ of the i[th] diagrid module link the shear force V_i and bending moment M_i to the module displacement Δu_i and rotation $\Delta\beta_i$, respectively. By applying compatibility, constitutive and equilibrium equations, $K_{T,i}$ and $K_{B,i}$ are obtained as follows:

$$K_{T,i} = 2N_w\left(\frac{A_{d,w,i}E}{L_d}\right)\cos^2\theta, \tag{1a}$$

$$K_{B,i} = N_f\left(\frac{B^2 A_{d,f,i}E}{2L_d}\right)\sin^2\theta\,, \tag{1b}$$

where N_w and N_f is the total number of diagonals in the web and flange façade, respectively, $A_{d,w,i}$ and $A_{d,f,i}$ the cross-sectional area of the web and flange members, E the elastic modulus of the diagonals and B the web dimension. The displacement Δu_i and rotation $\Delta\beta_i$ are equal to the product of the module height h and the shear and bending deformation, γ and χ, respectively. Specifying the desired values of shear and bending deformation, γ^* and χ^*, the member dimensions can be easily obtained as [10]:

$$A_{d,w,i} = \frac{V_i L_d}{2N_w E h\gamma^*\cos^2\theta}, \tag{2a}$$

$$A_{d,f,i} = \frac{2M_i L_d}{N_f B^2 E h\chi^*\sin^2\theta}. \tag{2b}$$

Since the horizontal load can act in either direction, the maximum value of the cross-sectional areas from Eqs. (2a-b) should be assigned to each diagonal which can act as either a web or flange member. The desired values of γ^* and χ^* are specified based on the desired deformation mode of the building. Assuming that the building sway mechanism is equivalent to the deformation of a cantilever beam, the lateral deflection at the top of the building *u(H)* can be written as follows:

$$u(H) = \gamma^* H + \frac{\chi^* H^2}{2}, \tag{3}$$

being $\gamma^* H$ and $\chi^* H^2/2$ the shear and bending contribution, respectively. In order to assess the relative contribution of bending versus shear deformation, Moon et al. [10] introduces a non-dimensional parameter s, given by the ratio of the bending to the shear contribution, i.e.:

$$s = \frac{\chi^* H^2}{2\gamma^* H}. \tag{4}$$

Combining Eqs. (3-4) and considering that the top lateral displacement is usually specified as a fraction of the total building height, i.e. $u(H) = H/\alpha$ (α usually being 500), one obtains the following relations between γ^*, χ^* and s:

$$\gamma^* = \frac{1}{(1+s)\alpha}, \tag{5a}$$

$$\chi^* = \frac{2\gamma^* s}{H} = \frac{2s}{(1+s)\alpha H}. \tag{5b}$$

Substituting Eqs. (5a-b) into Eqs. (2a-b), the member sizes can be obtained for the different values of the parameter s.

Adopting different s values leads to different preliminary sizing for the external diagonals. When s is extremely low, the shear deformation mode of the structure prevails over the bending mode and this leads to excessive material usage in the flange members to limit the bending deflection. Conversely, when s is high, the bending deformation prevails and the obtained cross-sectional areas are mainly governed by the web façades to limit the shear deformability. Therefore, an optimal value of s is shown to exist, s_{opt}, which balances the need to limit both shear and bending deformability [10]. In this case, the member sizes at the higher stories are usually governed by the shear deformation, while the ones at the lower stories are mostly controlled by the bending deformation. The s_{opt} depends on the building aspect ratio (H/B), and leads to the most efficient solutions that comply with the target maximum displacement while employing the minimum amount of material. For diagrid structures taller than 40 stories, with H/B greater than 5 and diagonal angles between 60° and 70°, the empirical equation $s_{opt} = H/B - 3$ is proposed [10].

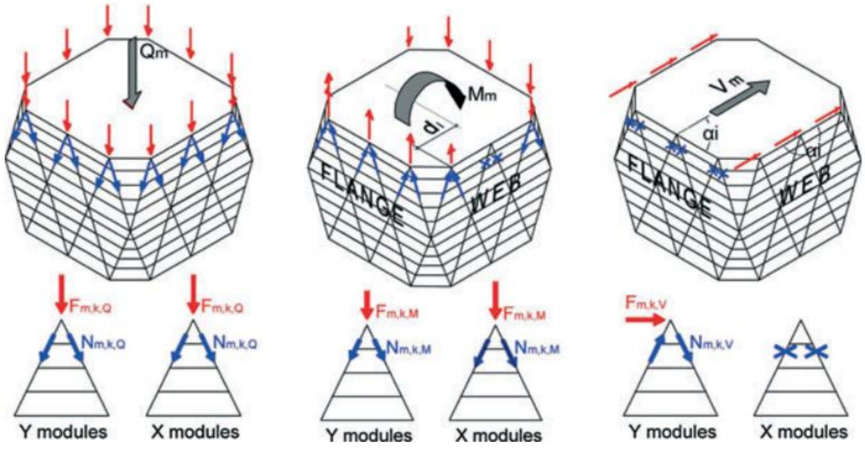
The other fundamental parameter that plays a key role in the preliminary design of diagrids is the diagonal inclination. Investigating a set of 20- to 60-story tall buildings, Moon et al. show that, for diagrid structures having aspect ratios of about 7, the optimal angle is between 65° and 75°, whereas for diagrids having aspect ratios of about 5 the optimal angle is lower than around 10° [10]. This is due to the competition between shear and bending stiffnesses in governing the deformation mode, and their dependence on the diagonal angle. Shear rigidity is maximum when the inclination is about 35°, while bending rigidity achieves its maximum value when the elements are vertical, i.e. θ = 90°. The optimal value to maximize both shear and bending rigidity lies between these two. Since shear mechanism prevails in shorter buildings and bending prevails in taller ones, it is expected that as the aspect ratio increases the building behaves more like a bending beam, thus the optimal angle increases [10]. This consideration has been strongly exploited in the analysis and design of diagrid systems, by considering various angle-based strategies and patterns to optimize the diagrid performance. More details about this subject are reported in Section 4.

The same stiffness-based approach reported in the previous paragraphs is also applied by Moon to braced tubes in [12]. In this case, the shear force is carried by the external mega-diagonals, while bending moment is carried by the perimeter vertical columns. Analyses based on 40- to 100-story tall braced tubes show that the optimal angle in this case is close to 45° and is less dependent on the building aspect ratio. This is due to the negligible involvement of external diagonals in carrying bending moment. For braced tubes with an aspect ratio greater than 6, Moon suggests a different empirical equation for the optimal s value, i.e. $s_{opt} = H/2B - 1$ [12]. It has to be noted that, in the same paper, the same analysis has been applied to diagrids with a broader range of heights than previously analyzed, i.e. from 40 to 100 stories. As a result, the author proposes a new empirical equation for the s_{opt} for diagrid structures with aspect ratios greater than 6, i.e. $s_{opt} = H/B - 2$.

In the cases investigated by Moon [10,12], it is found that the stiffness requirements drive the preliminary design and the strength criteria are usually fulfilled. Only a few members in the leeward façade of the building are found to fail when the maximum allowable displacement is increased, i.e. $\alpha < 500$. However, thanks to the high rigidity of the diagonalized façades, which make the diagrid structure highly efficient, strength requirements may be of paramount importance and in specific cases they might even govern the design criteria, as suggested by Montuori et al. in [13]. In this paper, a simplified strength-based methodology for the preliminary design of diagrid tubes is provided. Figure 5 shows the adopted scheme for the development of the strength-based approach. Both gravity and lateral loads are applied to the building.

Assuming that the internal core occupies the 25% of the floor area, the diagrid carries the 37.5% of the gravity load at the level of the mth module, Q_m (Figure 5a). This vertical loading condition generates a uniform compression state in all the n_k diagonals of the module, $N_{m,k,Q} = 0.375Q_m sin\theta/2n_k$. Lateral loads generate the bending moment M_m and shear force V_m at the module level. The former induces a uniform compression state in the diagonals of the leeward flange, a uniform tension state in the windward flange and a linear distribution of tension-compression axial forces in the webs, depending on the distance d_i of the ith diagonal from the center of the floor (Figure 5b). This leads to the expression of the axial force $N_{m,k,M} = \pm M_m d_k sin\theta/2\Sigma d_i$. Conversely, the shear force induces only tension-compression stresses in the web diagonals, therefore $N_{m,k,V} = \pm V_m cos\alpha_k cos\theta/2\Sigma cos\alpha_i$, being α the direction of the horizontal force with respect to the orientation of the diagrid module (Figure 5c).

Considering all the loading conditions, one obtains the total axial force in the generic diagonal, as $N_{m,k} = N_{m,k,Q} + N_{m,k,M} + N_{m,k,V}$. This value is finally used to define the member size, based on the tensile strength and the buckling compressive resistance of the diagonal. In the same paper, the authors also propose an analytical formulation, based on Euler-Bernoulli and Timoshenko beam theories, to obtain an alternative optimal s value for the stiffness-based approach, i.e. $s_{opt} = 0.19H^2/tan\theta L^2$.



(**a**) (**b**) (**c**)

Figure 5. Scheme of the elementary diagrid module for the definition of the strength-based approach for the diagrid preliminary design, under: (**a**) gravity loads; (**b**) overturning moment; (**c**) shear force. Used with permission from Montuori et al. [13].

The strength- and stiffness-based approaches are simultaneously applied for the preliminary design of a rectangular 100-story tall diagrid tube, considering three different diagonal angles (64°, 69° and 79°), under both gravity and wind loads. The results show that, on the broad side of the buildings, strength requirements always prevail at the upper modules, whereas stiffness criteria drive the design of the lower modules. Conversely, on the shorter side, strength prevails over stiffness for the entire height of the building with θ = 64°, stiffness prevails for θ = 79°, while in the case of 69° (which is close to the optimal angle inclination) the stiffness- and strength-based approaches provide almost the same result [13].

After carrying out the structural analyses on the designed buildings, it is found that the stiffness-based methodology leads to very efficient structures as regards the top lateral deflection, which is very close to the target value. However, this approach usually leads to unsatisfactory results in terms of inter-story drifts of the upper modules, as well as in terms of member strength demand-to-capacity ratio (DCR). In fact, besides the case of θ = 79°, where only 0.3% of the diagonals fail the strength requirements, 26% and 23% of them exhibit DCR greater than 1 for θ = 64° and 69°, respectively. On the contrary, adopting the strength-based design, the fraction of elements with DCR greater than 1 is 0%, 0.5% and 0.3%, for θ = 64°, 69° and 79°. However, with this approach, unsatisfactory results are obtained in terms of lateral deformability, especially in the case of 69° and 79° [13]. Therefore, stiffness-based approaches might lead to unsatisfactory strength results, while strength criteria might fail stiffness requirements. A compromise should then be found depending on the specific building characteristics. In both cases, large inter-story drifts are usually found at the upper modules. This issue has been thoroughly analyzed by Montuori et al. [11] and tackled by providing special internal systems, like SBSs. More details about this will be provided in Section 5.

Further investigation regarding the suitability of stiffness- and strength-based criteria for the preliminary design is subsequently developed to a broader range of diagrid structures [14,15]. In [14], Mele investigates the effect of both approaches on 90-story tall diagrid tubes, with diagonal angles of 60°, 70° and 80°. The results are in line with the previous findings. For smaller diagonal angles, strength usually drives the design, while stiffness-based approach leads to inadequate DCR values. For greater angles, stiffness-based design prevails, while strength criteria lead to excessive lateral deflections. In the range of the optimal angle, both criteria concur to define the member sizes.

More recently, the effect of both slenderness and diagonal angle has been taken into account simultaneously for the preliminary design [15]. Diagrids with aspect ratios ranging from 2 to 8 and diagonal angles from 50° to 80° are considered. It is found that, for aspect ratio from 2 to 4, the design is mainly governed by strength requirements, independently on the diagrid angle, and the "premium for height" is mostly linear with the increase of slenderness. Conversely, for aspect ratios greater than 6, the design is mainly driven by stiffness, and the weight increases more than linearly with the slenderness. Aspect ratios around 5 are found to be the threshold, where stiffness- and strength-based designs provide comparable solutions [15].

Based on these results, it is concluded that, due to the extreme rigidity of the diagrid tubular system, it is not always possible to know *a priori* whether stiffness- or strength-based criteria should be considered for the preliminary design. Both approaches are necessary and unavoidable, and none of them should be used without the other [13]. The geometrical diagrid parameters, e.g. building aspect ratio and diagonal angle, drive the prevalence of one over the other. In any case, simplified approaches for the preliminary design represent an effective way to quickly define and assess the structural characteristics and performance of the diagrid.

3. Methods for the structural analysis of diagrid tall buildings

In the academic literature, the most common procedure to deal with the structural analysis of diagrid systems is the Finite Element Method (FEM). However, simplified methodologies have also been developed for a quick evaluation of the overall diagrid structural behavior.

Mele et al. [16] have proposed a hand-based method for the evaluation of the axial stress in the diagrid members. The method is based on the analysis of the internal forces arising in the baisc triangular element due to gravity and vertical loads, taking also into account the effect of horizontal and vertical curvatures of the diagrid façade. Although it does not allow to calculate directly the displacements of the structure, this methodology is proven effective in the computation of the axial forces in the diagonals. It is applied to three real case studies, the Swiss Re Building (London), the Hearst Headquarters (New York) and West Tower (Guangzhou), and the axial stresses arising from hand-calculations show a very good correspondence with FEM results. Design considerations on the optimal diagonal inclination for the investigated cases are also provided.

More recently, Liu and Ma have developed a simplified methodology, called the modular method (MM), to perform the structural analysis of diagrid tubes with arbitrary polygonal shape [17]. So far, most of the research had been focused on rectangular diagrids, having vertical façades acting as webs or flanges (Figures 4-5); however, little attention was paid on diagrids with polygonal shapes.

The modular method relies on the modularization of the diagrid and the calculation of the lateral stiffness of the diagrid modules in order to compute the total lateral deflection. The lateral displacement u_i of the i[th] module can be obtained by superimposing the contribution of the shear displacement $u_{V,i}$ and bending displacement $u_{M,i}$. Based on the evaluation of the shear and bending rigidity of the i[th] module, $K_{V,i}$ and $K_{M,i}$, the two contributions can be computed as follows:

$$u_{V,i} = \frac{V_1}{K_{V,1}} + \frac{V_2}{K_{V,2}} + \cdots + \frac{V_i}{K_{V,i}}, \tag{6a}$$

$$u_{M,i} = \frac{M_1}{K_{M,1}} hi + \frac{M_2}{K_{M,2}} h(i-1) + \cdots + \frac{M_i}{K_{M,i}} h[i-(i-1)]. \tag{6b}$$

where V_i and M_i are the shear force and bending moment at the level of the i[th] module, respectively, and h the height of the module. The key of the MM is the calculation of the shear and bending rigidities, $K_{V,i}$ and $K_{M,i}$, and is based on the usual assumptions for diagrid tubes: the diagonals are only subjected to axial stress and remain in the linear elastic regime, the building floors behave as rigid bodies without any internal deformation, the intra-module floors are neglected for the calculations of the modular rigidities.

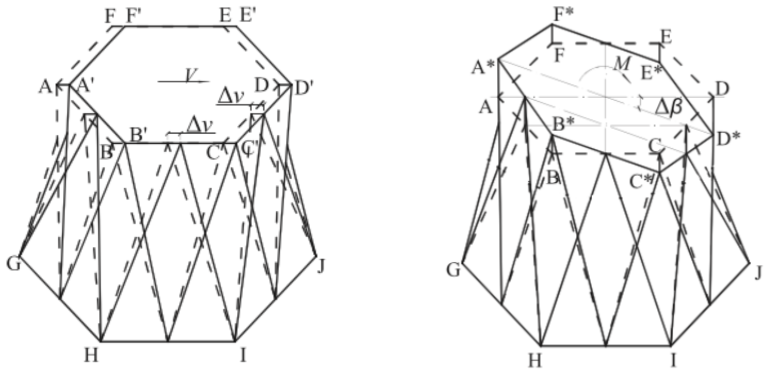
Shear rigidity is defined as the total shear force F_V required for unitary horizontal displacement of the module Δv (Figure 6a), and bending rigidity is defined as the bending moment M required for unitary floor rotation $\Delta\beta$ (Figure 6b). Applying independently unitary floor displacements and rotations, and computing the total shear force and bending moment, leads to the direct evaluation of $K_{V,i}$ and $K_{M,i}$. The calculation of the shear force and bending moment is based on the geometrical compatibility equations, the constitutive relations of the diagonals and finally the equilibrium equations at the level of the floor. This finally allows to obtain the following formulations for K_V and K_M:

$$K_V = \frac{EA\cos^2\theta\sin\theta\sin\gamma}{h}\sum_{d=1}^{N}\cos^2\alpha_d + \frac{EA\sin^3\theta\cos^2\gamma\sin\gamma}{h}\sum_{d=1}^{N}\sin^2\alpha_d, \tag{7a}$$

$$K_V = \frac{EA\sin^3\theta\sin^3\gamma}{h}\sum_{d=1}^{N}{B_d}^2\,, \tag{7b}$$

where E and A are the Young modulus and cross-sectional areas of the diagonals, θ the angle between the diagonal and the main ring beam in the façade, γ the angle between the ring beam plane and the façade, N the number of total diagonals in the module, α the angle between the ring beam and shear direction and B_d is the distance between the diagonal d and the neutral axis in the main ring beam plane [17]. Note that Eqs. (7a-b) resemble Eqs. (1a-b), but they also include the effect of not-vertical façades (γ angle) and polygonal planar shapes (α angle). Making use of Eqs. (7a-b) for each module, together with the application of Eqs. (6a-b), one can finally evaluate the lateral deformation of the diagrid building under horizontal loads.

The MM is verified against FEM calculations, analyzing square, hexagonal and octagonal diagrid tubes with vertical and inclined façades under different horizontal loading conditions. The variations in terms of top displacements from FEM results are always within 10%, which verifies the proposed methodology. Based on the evaluation of the shear and bending rigidities, the MM is also employed to define the analytical framework for the preliminary design of diagrids [17].



(**a**) (**b**)

Figure 6. Scheme for the calculation of (**a**) shear rigidity; (**b**) bending rigidity, according to the modular method (MM). Used with permission from Liu and Ma [17].

Subsequently, a new method has been developed for the structural analysis of diagrids by Lacidogna et al. in 2019 [18], which has been called the matrix-based method (MBM). This methodology is grounded on matrix calculus and is similar to the FE method, although it drastically reduces the degrees of freedom (DOFs) of the diagrid structure. It is based on the same assumptions employed by the previous authors [10,17]: the diagonals carry only axial force and remain in linear elastic regime, the intra-module floors are neglected and the building deformation obeys the plane section assumption. The major reduction in the system DOFs, compared to conventional FE models, is due to the fact that, under the above assumptions, the considered DOFs in the MBM are only the displacements and rotations of the rigid floors, rather than the nodal displacements and rotations associated to all the structural elements.

The structural problem for the 3D free-form diagrid tubes is formulated through the generalized Hooke's law as $\{F\} = [K]\{\delta\}$, $\{F\}$ and $\{\delta\}$ being the force and displacement vectors, respectively, and $[K]$ the stiffness matrix of the diagrid. Considering a number of floors equal to N, the total dimension of the structural problem is $6N \times 6N$, being six the number of DOFs per floor, i.e. three translations and three rotations. The matrix equation can be expanded as follows, where all the DOFs contributions are highlighted:

$$\begin{Bmatrix} \{F_x\} \\ \{F_y\} \\ \{M_z\} \\ \{M_x\} \\ \{M_y\} \\ \{F_z\} \end{Bmatrix} = \begin{bmatrix} [K_{F_x\delta_x}] & [K_{F_x\delta_y}] & [K_{F_x\varphi_z}] & [K_{F_x\varphi_x}] & [K_{F_x\varphi_y}] & [K_{F_x\delta_z}] \\ [K_{F_y\delta_x}] & [K_{F_y\delta_y}] & [K_{F_y\varphi_z}] & [K_{F_y\varphi_x}] & [K_{F_y\varphi_y}] & [K_{F_y\delta_z}] \\ [K_{M_z\delta_x}] & [K_{M_z\delta_y}] & [K_{M_z\varphi_z}] & [K_{M_z\varphi_x}] & [K_{M_z\varphi_y}] & [K_{M_z\delta_z}] \\ [K_{M_x\delta_x}] & [K_{M_x\delta_y}] & [K_{M_x\varphi_z}] & [K_{M_x\varphi_x}] & [K_{M_x\varphi_y}] & [K_{M_x\delta_z}] \\ [K_{M_y\delta_x}] & [K_{M_y\delta_y}] & [K_{M_y\varphi_z}] & [K_{M_y\varphi_x}] & [K_{M_y\varphi_y}] & [K_{M_y\delta_z}] \\ [K_{F_z\delta_x}] & [K_{F_z\delta_y}] & [K_{F_z\varphi_z}] & [K_{F_z\varphi_x}] & [K_{F_z\varphi_y}] & [K_{F_z\delta_z}] \end{bmatrix} \begin{Bmatrix} \{\delta_x\} \\ \{\delta_y\} \\ \{\varphi_z\} \\ \{\varphi_x\} \\ \{\varphi_y\} \\ \{\delta_z\} \end{Bmatrix}. \quad (8)$$

In Eq. (8) $\{F_x\}$, $\{F_y\}$, $\{F_z\}$ represent the forces acting at the floor level on the horizontal (X, Y) and vertical (Z) direction, respectively, and $\{\delta_x\}$, $\{\delta_y\}$, $\{\delta_z\}$ are the corresponding displacements. $\{M_x\}$ and

$\{M_y\}$ contain the out-of-plane moments acting along the X and Y directions, respectively, while $\{\phi_x\}$ and $\{\phi_y\}$ are the corresponding out-of-plane rotations. Finally, $\{M_z\}$ and $\{\phi_z\}$ denote the torque moments and rotations acting around the vertical axis. Based on the expansion of the force and displacement vectors reported in Eq. (8), the $6N \times 6N$ stiffness matrix is partitioned accordingly, where each $N \times N$ submatrix relates a force-moment vector to the generic displacement-rotation vector. The procedure for the calculation of the stiffness matrices is grounded on the displacements method, and is similar to the scheme adopted by Liu and Ma in the MM [17]. The stiffness coefficients are obtained by applying unitary displacements-rotations at the floor levels and evaluating the total reacting forces-moments according to compatibility, constitutive and equilibrium equations.

The MBM is more general than the MM, since it does not consider only shear and bending rigidities (contained in the matrices $[K_{Fx,\delta x}]$, $[K_{Fy,\delta y}]$, $[K_{Mx,\phi x}]$ and $[K_{Mx,\phi x}]$), but also the vertical and torsional ones, i.e. $[K_{Fz,\delta z}]$ and $[K_{Mz,\phi z}]$. Besides these 6 sub-matrices that lie on the diagonal of the stiffness matrix, the MBM also evaluates other 30 mixed submatrices, although only 15 of them need to be computed due to the symmetry properties of $[K]$. For regular-form diagrids most of these out-of-diagonal matrices are zero; nevertheless, the evaluation of the matrices $[K_{Fx,\phi x}]$, $[K_{Fy,\phi y}]$, $[K_{Mx,\delta x}]$ and $[K_{Mx,\delta y}]$ is extremely important, as they contain the information about the coupling between shear and bending stiffnesses, therefore concurring to the correct definition of the lateral deflection.

After the complete calculation of the stiffness coefficients of the 21 non-identical submatrices in Eq. (8), the application of forces and moments at the floor levels leads to the evaluation of the corresponding displacements and rotations, through the inversion of Eq. (8). Eventually, known the deformation of the structure, the compatibility and constitutive equations are applied once again, in order to find the axial forces in the diagonals. The MBM is applied to perform the structural analysis of the double-curvature Swiss Re Tower and comparisons with FEM calculations show the consistency of the suggested method for the evaluation of both lateral and vertical displacements, as well as torsional rotations [18].

The MBM has not only been developed for the structural analysis of diagrids, but also to investigate the interaction between a diagrid tube and other resisting elements embedded within the building. To do so, the MBM has been built within the framework of the General Algorithm (GA), a matrix-based analytical methodology proposed in 1985 for the preliminary analysis of tall buildings [19]. The GA was firstly developed for the analysis of 3D civil buildings with moment resisting frames and closed-section shear walls. Further on, open-section shear walls were also taken into account [20], which observe the Vlasov's theory of deformation and exhibit the warping effects typical of thin-walled structures [21]. In recent years, the GA has also allowed to study the interaction between structures of different heights [22], secondary effects in tall buildings [23], unconventionally-designed systems such as tapered and twisted towers [24]. It has also been applied to investigate the dynamic behavior of tall buildings [25], as well as real case studies in Northern Italy [26,27].

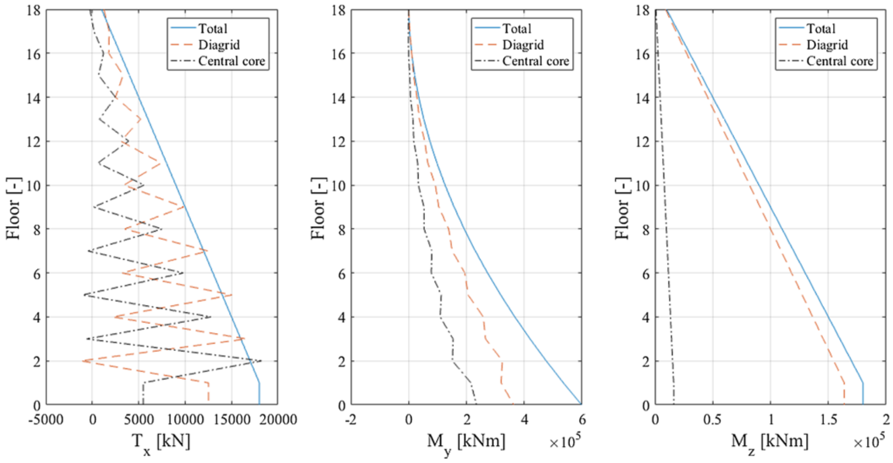
The framework of GA takes into account only 3 DOFs per floor, namely the two horizontal displacements and one torsional rotation. To make the MBM suitable for insertion into the GA, Lacidogna et al. [18] carry out a static condensation procedure, where the contributions of vertical displacements and out-of-plane rotations are condensed. Specifically, Eq. (8) is re-written in the following form:

$$\begin{Bmatrix} \{F_H\} \\ \{F_V\} \end{Bmatrix} = \begin{bmatrix} [K_{HH}] & [K_{HV}] \\ [K_{VH}] & [K_{VV}] \end{bmatrix} \begin{Bmatrix} \{\delta_H\} \\ \{\delta_V\} \end{Bmatrix}, \quad (9)$$

where $\{\delta_H\}$ ($\{F_H\}$) include the contributions of horizontal displacements (forces) and torsional rotations (moments), and $\{\delta_V\}$ ($\{F_V\}$) include the contribution of vertical displacements (forces) and out-of-plane rotations (moments). The condensation procedure yields the following relation, where only the horizontal DOFs are considered through the $3N \times 3N$ condensed stiffness matrix $[K_{HH}]^*$:

$$\{F_H\} = ([K_{HH}] - [K_{HV}][K_{VV}]^{-1}[K_{VH}])\{\delta_H\} = [K_{HH}]^{*}\{\delta_H\}. \quad (10)$$

The MBM has been used within the GA framework to investigate the stiffness interaction between an external steel diagrid and an internal concrete core. In particular, a square 18-story tall building is considered in [18] and the coupled behavior is analyzed under lateral forces and torque moments. Although the torsional behavior is obviously governed by the external diagrid tube, the distribution of shear forces at the various floor levels is not trivial and gives rise to an oscillating trend along the height of the building, due to the shear-bending coupling of the two structural systems (Figure 7).



(**a**) (**b**) (**c**)

Figure 7. External steel diagrid tube coupled with a central concrete core. Distribution of: (**a**) shear forces; (**b**) bending moments; (**c**) torque moments, according to the MBM and GA. Used with permission from Lacidogna et al. [18].

In a more recent paper, Lacidogna et al. [28] investigate the effect of the diagonal inclination on the diagrid-core coupled system. As already pointed out by Moon et al. [10], when the diagrid angle is in the optimal range, the diagrid lateral stiffness prevails over the internal core's one. The influence of the type of internal core, i.e. closed- or open-section shear wall, is also investigated in [28]. Although the diagrid-core coupling mechanism is almost the same in terms of the lateral deformability, a remarkable difference between the two types of internal cores (open- and closed-section shear wall) is observed in terms of torsional behavior. In the case of internal open-section shear wall and steep diagrid angles, a clear inflection point in the torsional deformation curve is obtained due to the warping effect of the internal shear wall [28].

Although simplified, the methods presented in this section for the structural analysis of diagrid systems, integrated with the stiffness- and strength-based methodologies for the preliminary design shown in Section 1, can provide a valid and efficient alternative to FE calculation in the preliminary stages. As a matter of fact, they allow the quick investigation of the overall structural behavior, while capturing the fundamental parameters governing the diagrid performance. It has to be noted that, due to the increasing power of nowadays computing technologies, simplifying the structural model, i.e. reducing the DOFs of the system, is not often an imperative need. Current FE software in modern computers are able to perform the structural analysis of buildings with very large number of DOFs, and the trend nowadays is to consider even more detailed models for the sake of general analysis. However, although using detailed models is necessary in the ultimate design stages, it can make lose sight of the overall structural behavior during the preliminary stages. As a matter of fact, in these stages, the correct comprehension of the overall structural behavior has important implications on the definition of the optimal resisting elements. These choices in the preliminary phases are in turn known to have a strong influence on the cost and efficiency of the final solution, especially in tall buildings. For this reason, using simplified methodologies for the preliminary design can help the designer to acquire awareness on the overall structural behavior, that the application of very detailed FE models might not reveal at first sight.

4. Optimization of the diagrid performance based on the geometrical features

Besides the great stiffness of the diagonalized façades and the capability to realize complex-shaped systems, one of the main successful aspects of diagrids is the possibility to reach high structural performance thanks to the optimization of the geometrical features. In the last decade, various researchers have thoroughly investigated the structural behavior of diagrids upon changing the external diagonal pattern, in order to reach the optimal solutions.

Moon et al. [10] show for the first time that there exists a diagonal angle capable of satisfying the stiffness requirements with the minimum amount of employed material. The optimal angle results from the need to limit both shear and bending deformation, and it is found to increase as the aspect ratio of the building increases. As already remarked in Section 2, for 60-story tall diagrids having an aspect ratio of about 7, the optimal angle is in the range 65°–75°, while it decreases of about 10° for aspect ratios close to 5 [10]. Approximately the same results are found in [12] for a set of 40- to 100-story tall diagrids.

Under lateral actions, shear forces and bending moments have different trends along the building elevation. For example, if we consider a uniform horizontal load, the shear force is zero at the top of the building and increases linearly towards the base, while the bending moment increases quadratically. This means that the need to resist shear and bending actions is different in different parts of the structure. Shear force prevails in the upper portion, while bending moment drives the design of the lower part. Based on this consideration, Moon investigates diagrid buildings with different patterns of diagonal angles [12]. Figure 8a shows a varying-angle diagrid with steeper angles at the base, Figure 8b a uniform-angle diagrid, and Figure 8c a varying-angle diagrid with steeper angles at the top. Steeper diagonals are more suitable for carrying bending moment, while shallower diagonals are more appropriate to carry shear force. Therefore, the solution in Figure 8a should enhance the structural performance of the diagrid. Conversely, the solution with steeper diagonals at the top behaves against structural logics and is only considered for sake of completeness, as it is not supposed to provide any beneficial effect.

Based on the results, it is found that, for shorter buildings with aspect ratio lower than 7, the uniform-angle configuration provides the most efficient design in terms of material consumption. Shorter buildings behave like shear beams and, while the steeper diagonals at the base enhance the bending stiffness, the negative effect of the reduced shear rigidity causes the varying-angle diagrids to lose efficiency. Contrariwise, for taller buildings with aspect ratio greater than 7, the bending behavior prevails. The reduced shear rigidity at the base due to the steeper diagonals is balanced by the significant increase in bending stiffness. Therefore, in this case, the varying-angle configuration is found to provide the most efficient solution [12]. The same results are found in another paper by Moon [30], where the author takes also into account the “speed” of variation of the diagonal angles along the height of the building, with smooth or more radical changes.

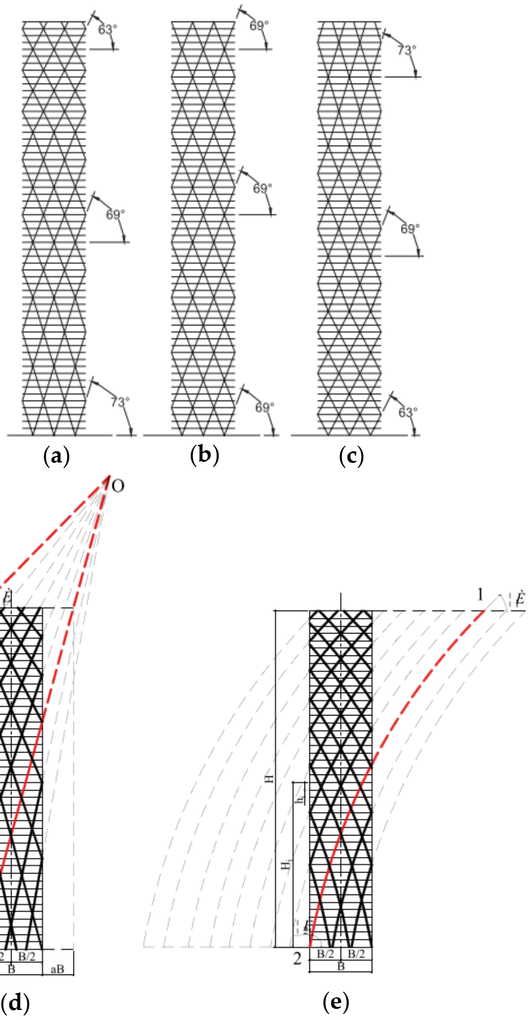


Figure 8. Different diagonal angle patterns diagrids: (**a**) varying-angle with steeper diagonals at the base; (**b**) uniform-angle; (**c**) varying-angle with steeper angle at the top, used with permission from Moon [12]; (**d**) varying-angle with straight diagonals; (**e**) varying-angle with curved diagonals, used with permission from Zhao and Zhang [29].

In the solutions provided by Moon with variable angles, the diagonals do not remain straight in their length over the full height of the building, because of their changing direction at the interface of two diagrid modules with different angles. To overcome this, Zhang et al. [31] propose a different strategy for the generation of varying-angle diagrid tubes. As shown in Figure 8d, a graphic approach is suggested to generate a varying-angle pattern with straight diagonals that extend over the full height of the building. This pattern is governed by two fundamental parameter, the top angle θ_1 and the bottom angle θ_2. The stiffness- and strength-based design criteria are applied to a set of 30- to 75-storey tall varying-angle diagrids with straight diagonals, with aspect ratios ranging from 3.6 to 9. Several θ_1 – θ_2 combinations are considered to investigate the optimal solutions under gravity and wind loads. Based on the results, the following empirical formulas are suggested for the optimal values of θ_1 and θ_2, depending on the building aspect ratio H/B:

$$\theta_{1,opt} = \begin{cases} \theta_{2,opt}, & H/B \leq 3.5 \\ \dfrac{1}{\left(1+\ln\dfrac{H/B}{3.5}\right)^{\frac{H/B}{2}}}\left(\theta_{2,opt} - \arcsin\dfrac{1}{\sqrt{3}}\right) + \arcsin\dfrac{1}{\sqrt{3}}, & H/B > 3.5 \end{cases}, \quad (11a)$$

$$\theta_{2,opt} = \arctan \frac{H/B}{1 + 0.475\sqrt{\frac{H/B}{4.75}}}. \tag{11b}$$

As H/B increases, the optimal bottom angle $\theta_{2,opt}$ increases, while the optimal top angle $\theta_{1,opt}$ decreases. A critical value of the aspect ratio, $(H/B)_{crit}$ is found, which defines the interface between the efficiency of uniform- versus varying-angle diagrids, meaning that below $(H/B)_{crit}$ uniform-angle diagrids are more efficient, while above this value varying-angle structures provide the most economical solutions. In this paper, $(H/B)_{crit}$ is found to be 4.5-5, smaller than the value of 7 previously suggested by Moon [12,30]. This is mainly due to the different definition of the diagonal pattern. For aspect ratios less than $(H/B)_{crit}$, the bottom angle rather than the top angle drives the design. Conversely, for greater aspect ratios, the top angle becomes one of the determining factors [31].

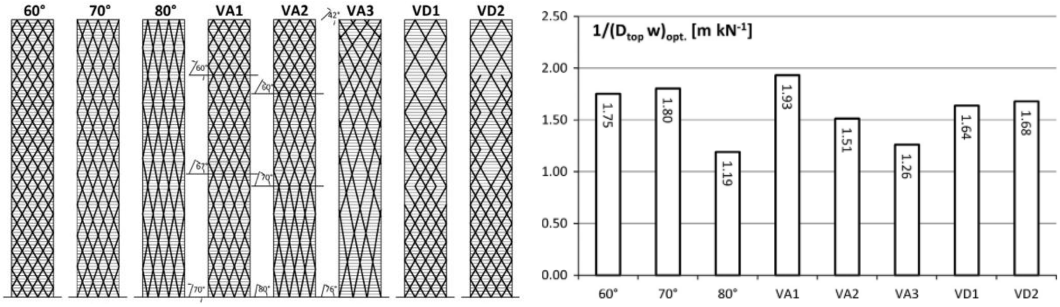
In a following paper, Zhao and Zhang [29] propose an additional diagrid configuration, where the varying-angle solution is obtained with curved diagonals (Figure 8e). In the same paper, they also consider seismic loads for the evaluation of the optimal diagrid pattern. It is found that, for varying-angle straight diagonals, the optimal bottom angle $\theta_{2,opt}$ is not affected by the load type, thus Eq. (11b) holds also for seismic loads. Whereas, the optimal top angle $\theta_{1,opt}$ is always close to the lower limit for seismic loads, i.e. $\theta_{1,opt} = \mathbf{arcsin\ 1/\sqrt{3}}$, thus correcting Eq. (11a). In the case of diagrids with curved diagonals, they propose the following equations for the optimal angles, which are valid for both wind and seismic loads:

$$\theta_{1,opt} = 0.8\left(\frac{H/B}{8}\right)^{\frac{1}{8}}\theta_{2,opt}, \tag{12a}$$

$$\theta_{2,opt} = \arctan(H/B), \tag{12b}$$

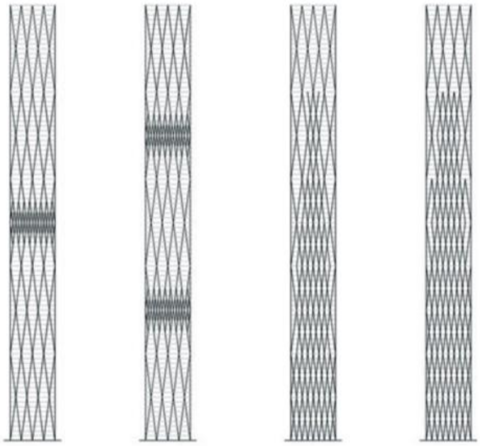
H/B being in the range 3.6 – 9. With these values, the optimal top angle $\theta_{1,opt}$ lies in the range 50°–70°, greater than the top angles in diagrids with straight diagonals (35°–45°). Thus, the smaller difference between $\theta_{1,opt}$ and $\theta_{2,opt}$ in this case results in a small curvature of the diagonals [29].

Further developments in the external diagrid patterns are carried out by Montuori et al. [32]. In addition to the consideration of uniform- and varying-angle (VA) solutions, the authors also propose diagrid patterns with variable density (VD) in the diagonal layout (Figure 9a). FE calculations are performed on a 90-storey tall building with aspect ratio of 6.62, under gravity and wind loads, and the structural responses are analyzed in terms of top lateral deflection, inter-story drifts and diagonals DCR. For each solution, an efficiency parameter is proposed as $1/D_{top}w$, D_{top} and w being the top lateral displacement and the employed steel weight per total floor area. The lower the lateral displacement and the amount of steel, the greater the efficiency of the investigated solution. The obtained efficiency parameters are shown in Figure 9b for all the considered solutions. From the results, it is found that the 80° and VA3 solutions result the less efficient for the investigated case, whereas VA1 is the most efficient one. Uniform-angle solutions with 60° and 70°, as well as the variable patterns VA2, VD1 and VD2, show similar efficiency values [32].



(**a**) (**b**)

Figure 9. (**a**) Different geometrical patterns from Montuori et al. [32]: uniform-angle patterns (60°, 70°, 80°), varying-angle patterns according to Moon approach (VA1, VA2) [12,30], varying-angle pattern according to Zhang approach (VA3) [31], variable-density patterns (VD1, VD2). (**b**) Efficiency parameters for the investigated solutions. Used with permission from Montuori et al. [32].

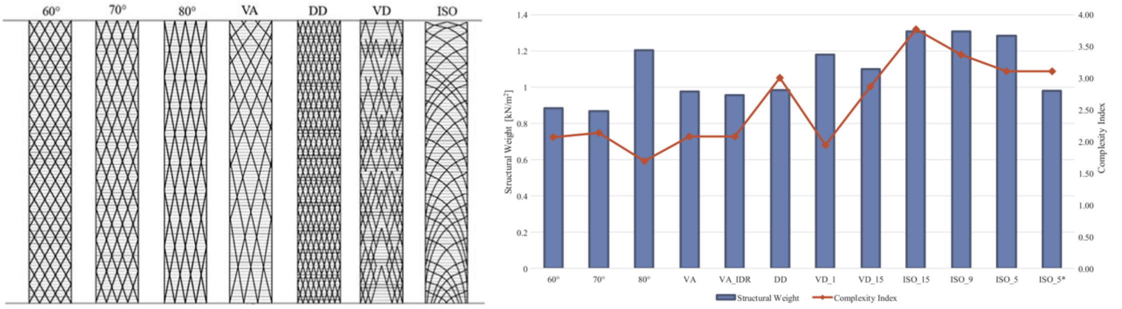


(**a**) (**b**) (**c**) (**d**)

Figure 10. Variable-density (VD) patterns proposed by Angelucci and Mollaioli [9]: (**a**-**b**) concentrated outrigger-like VD pattern; (**c**-**d**) distributed VD pattern. Used with permission from Angelucci and Mollaioli [9].

Additional analyses regarding different pattern configurations can be found in the work of Angelucci and Mollaioli [9]. After exploring the effectiveness of stiffness-based approaches for a 351-meter tall diagrid with optimal (69°) and non-optimal (82°) diagonal angle in order to evaluate whether common approaches lead to optimized member sizes, the authors propose additional variable-density (VD) patterns of the diagonal arrangement (Figure 10). Two VD strategies are suggested for the non-optimal (82°) diagrid tube to meet the stiffness requirements: a localized pattern, resembling one-outrigger-like (Figure 10a) or two-outrigger-like (Figure 10b) schemes; a more uniform VD pattern, which provides distributed additional stiffness over the building elevation (Figures 10c-d). The outcomes from FE calculations show that the local density increments (Figures 10a-b) are not efficient strategies to meet stiffness and strength requirements. Conversely, the solutions involving a more uniform VD pattern (Figures 10c-d), where the diagonal concentration rarefies towards the top of the building, turn out to be appropriate solutions to limit the lateral displacements, while obtaining notable material savings [9].

The previous work of Montuori et al. [32] has been subsequently developed by Tomei et al. [33], who propose additional diagonal patterns for the 90-story tall diagrid building (Figure 11a). Besides considering the usual uniform- and varying-angle patterns, the authors also suggested a double-density pattern (DD), where the diagonal layout is doubled and mirrored over the diagrid façade, a variable-density pattern (VD), generated starting from the DD pattern with further topology optimization, and a diagrid-like pattern (ISO), where the diagonals follow the principal stress lines obtained from the equivalent building cantilever. Stiffness- and strength-based preliminary designs are carried out, together with optimization procedures based on Genetic Algorithms through the use of commercial software. The optimization procedure aims at minimizing the unit structural weight of the building, while complying to the stiffness and strength requirements. This is achieved by formulating an objective function (OF) to be minimized and specifying the constraints of the optimization problem, as thoroughly described in [15,33].



(**a**) (**b**)

Figure 11. (**a**) Geometrical patterns for the 90-story tall diagrid building considered by Tomei et al. [33]: uniform-angle patterns (60°, 70°, 80°), varying-angle pattern according to Zhang approach (VA) [31], double-density pattern (DD), variable-density pattern (VD), stress lines pattern (ISO). (**b**) Unit structural weight (blue bars) and complexity index (red curve) for the investigated diagrid patterns. VA_IDR, VD_1, VD_15, ISO_15, ISO_9, ISO_5, ISO_5* refer to additional subsets of the corresponding patterns, as reported in [33]. Used with permission from Tomei et al. [33].

The results of the analysis are analyzed in terms of unit structural weight, diagonal cross-section distribution along the elevation, deformed configuration, lateral displacements, inter-story drifts, diagonal DCR, highlighting the most efficient solutions from the structural viewpoint. The authors also propose a complexity index, which accounts for the "constructability" of the diagrid structure. This is defined taking into account five main metrics, i.e. the total number of nodes, the number of different cross-sections, the number of diagonal splices necessary for transportation purposes, the total number of diagonals and the number of different diagonal lengths. The results of the complexity index, together with the obtained structural weight, are shown in Figure 11b for each geometrical pattern. Graphs like the one reported in Figure 11b can be extremely useful for evaluating both the structural efficiency and constructability of the investigated diagrid solutions [33].

The analyses shown in the previous paragraph of this Section, for the assessment of diagrid performance, take into account only square and rectangular buildings. To consider also different plan shapes, Mirniazmandan et al. [34] recently investigate the simultaneous effect of diagonal inclination and planar shape on the top lateral displacement and diagrid weight. Sixty-four parametric models of a 180-meter tall building, with various cross-sectional shape, are generated by randomly increasing the number of sides at both the base and top plans. Five diagonal angles are also considered, in the 33°–81° range. By means of Genetic Algorithms coupled with FE structural analyses, the authors find out that the diagonal angle of 63° provides the least amount of top lateral deflection, while reducing the employed structural material. Furthermore, it is found that increasing the sides of the base and top plans leads to the most efficient solutions in terms of lateral displacements, although the increase of structural performance is not as evident as when changing the diagonal inclination.

More recently, a similar analysis has been carried out by Lacidogna et al. [35], to investigate the influence of both the diagonal inclination and plan shape on the structural behavior of diagrid tubes. In this study, the structural response for square, hexagonal, octagonal and circular diagrids is evaluated not only in terms of lateral deflection, but also in terms of torsional rotations. The analysis is carried out by means of the previously developed matrix-based method (MBM) [18], and shows that the diagonal angle is the main parameter governing the structural response rather than the specific plan shape. As already found out previously by Moon et al. [10], the optimal angle to minimize the lateral displacement increases with the aspect ratio of the building, as it results from the need of limiting both shear and bending deformability. Conversely, the optimal angle to minimize torsional rotations is always found to be the shallowest one, close to 35°, and it does not depend on the building aspect ratio. This is mainly due to the different mechanisms which drive the lateral and torsional deformability of the diagrid. As already reported, the former is affected by both the shear and bending stiffness of the diagrid modules, whereas only the shear rigidity concurs in the definition of the torsional behavior. Due to the fact that shear rigidity is maximum for shallower diagonal angles, these are the most effective to resist torque moments [28,35]. Therefore, when torque actions need to be taken into account, this aspect must be considered in the definition of the optimal grid pattern.

Finally, all the analyses presented so far have mainly dealt with tubular structures with vertical façades. In a very recent paper, Ardekani et al. [36] investigate the influence of the plan shape, together with the convexity and concavity of the diagrid surface (Figure 12). Based on FE calculations on a set of 40-story tall buildings, the outcomes show that, compared to rectangular diagrids, other polygonal forms might lead to beneficial material savings, while meeting the stiffness requirements. Furthermore, with respect to the normal models, the buildings with convex and concave façades achieve better results in terms of structural performance.

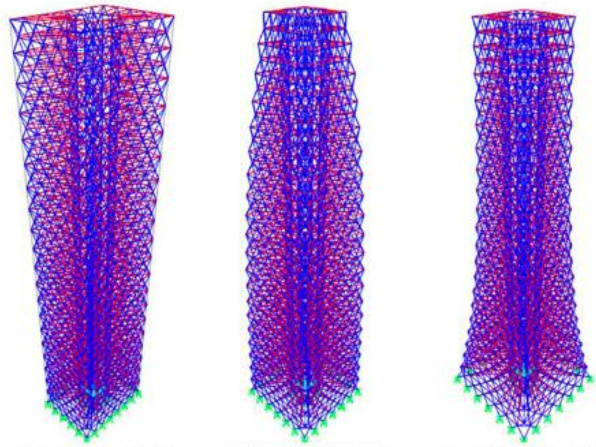


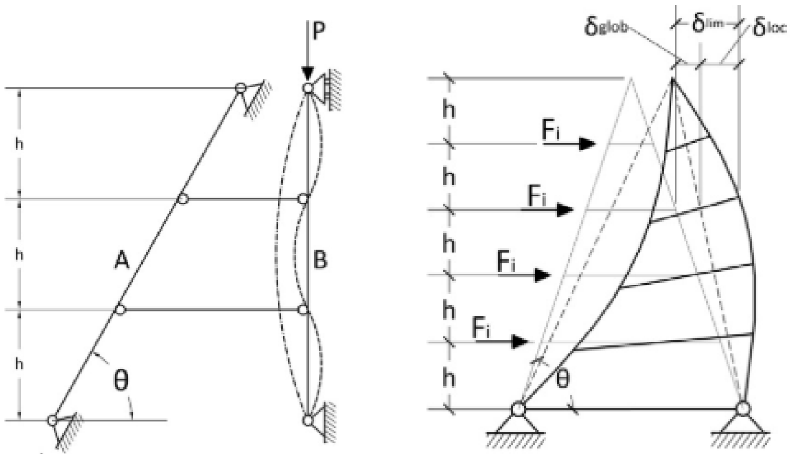
Figure 12. Diagrid structures with vertical, convex and concave façades. Used with permission of Taylor & Francis Ltd (www.tandfonline.com), from Ardekani et al. [36].

As can be easily recognized from the studies reported in this Section, one of the main aspects that has caused the notable diffusion of diagrids in recent years is related to the versatility of its external diagonal layout. A rational and optimized diagonal pattern allows to achieve remarkable structural performance, together with beneficial material savings. The application of expeditious FE calculations, as well as simplified methodologies such as the ones reported in the previous Sections, together with optimization techniques, can help engineers and designers to reach high-performance structures in the preliminary stages of the tall building design.

5. Strategies for tackling excessive inter-story drifts and stability phenomena in diagrids

In the previous Section, we have seen that the external diagonal layout can be properly modified to meet the necessary stiffness and strength requirements. Accordingly, the external mega-bracings can extend over multiple stories. As pointed out by Montuori et al. [11], this can give rise to two local structural issues which need to be carefully addressed by the designer: (a) the instability of interior columns and (b) excessive inter-story drifts. Both of them are mainly due to the lack of flexural resistance of the diagonals. This section investigates these local issues, as reported in the fundamental work of Montuori et al. [11].

The first local issue is shown in Figure 13a, where a 3-story diagrid module is represented. Element A and B represents the external diagonal and the interior column, respectively. The column usually extends over the full height of the building and it is subjected to high compression forces due to gravity loads, which might induce Eulerian buckling. The column's resistance to lateral buckling mode relies on the external diagrid structure, which fully braces the interior column only at the panel points. Within the module height, the multi-story buckling mode is only prevented by the flexural resistance of the diagonals. If this is not enough, the multi-story sway mechanism takes place, which can occur at lower buckling loads than the one-story mode.



(**a**) (**b**)

Figure 13. Local issues in the design of diagrid tall buildings: (**a**) stability of interior columns; (**b**) excessive inter-story drift of intra-module floors. Used with permission from Montuori et al. [11].

Based on a simplified analytical formulation, Montuori et al. [11] propose a simple equation in order to check whether the flexural resistance of the diagonals is sufficient to the purpose, specifically:

$$n_{dg} I_{dg} > (k^2 - 1) \sin\theta \, n_{col} I_{col}, \tag{13}$$

n_{dg} and I_{dg} being the number of diagonals along the perimeter and their inertia moment, respectively, k the number of intra-module stories, θ the diagonal inclination, n_{col} and I_{col} the number and inertia moment of the gravity columns. If Eq. (13) is not satisfied, the internal columns buckle in a multi-story sway mode (Figure 13a). In this case, either the columns are designed to sustain greater buckling loads or a secondary system is necessary.

The second local issue is related to the excessive inter-story drifts of intra-module floors. As in the case of the column stability, the lateral displacements of these floors rely on the flexural resistance of the mega-diagonals. Based on the scheme reported in Figure 13b, a simple expression is obtained by Montuori et al. to assess the need of additional systems for the limitation of inter-story drifts [11]:

$$\frac{\left(\sum_{i=1}^{k-1} F_i\right) \sin\theta \, L^2}{24 E I_{dg} n_{dg}} < \frac{500 - \alpha}{500\alpha}, \tag{14}$$

F_i being the horizontal force applied at the i^{th} intra-module floor, L the total span of the diagonal, E the elastic modulus, α the limiting factor for the inter-story drift (usually α = 300), and k, I_{dg} and n_{dg} with the same meaning reported above. If Eq. (14) is not satisfied, either the inertia moment of the diagonals I_{dg} is increased or, again, an additional structural system is needed.

Since the diagonals in diagrid systems are usually designed to carry only axial load, their flexural resistance is often not enough to prevent the multi-story sway mode of interior gravity columns, as well as the excessive inter-story drifts of the intra-module floors. For this reason, Montuori et al. [11] propose the adoption of a secondary bracing system (SBS), realized with limited modifications to the simple frames of the interior service core. Either rigid connections at beam-column joints or triangulation of the structural framework are proposed, thus obtaining a moment resisting frame (MRF) or concentric braced frame (CBF) respectively. The design of the SBS is carried out to address the lack of stability of the interior columns, the excessive inter-story drifts, or both.

Under the authors' assumptions, the first issue gives rise to a force in the bracing system F_{br} equal to $0.004P_{cr,col,NS}$, $P_{cr,col,NS}$ being the buckling load of the fully braced column. This force can be directly employed to design the members of the SBS, e.g. the diagonals of the CBF, for the stabilization of the internal gravity columns. Similarly, the second local issue is tackled with the design of an SBS able to provide a required lateral stiffness $\beta_{req,d}$ equal to $250\alpha(\Sigma F_i)/(500h\text{-}\alpha h)$ [11].

In the paper, the authors analyze a 90-story tall diagrid building, with diagonal angles equal to 60°, 70° and 80°, to test the efficacy of the proposed formulation for the SBS. Application of Eq. (13) shows that the 70° and 80° buildings have almost all the diagonal members with inertia less than the minimum required for the stability of internal columns, whereas in the 60° case only the upper diagrid modules are able to provide enough resistance against the multi-sway mode. This is mainly due to the lower number of intra-module floors in the 60° solution. Thus, a SBS is found to be necessary to stabilize interior columns. Similarly, the application of Eq. (14) reveals that, in all cases, SBSs are needed to limit the excessive inter-story drifts at the upper modules.

For this reason, SBSs are designed consisting of four CBFs, to both stabilize interior columns and comply with the imposed drift limitation (α = 300). In Figure 14, the results are shown for the three building solutions in terms of lateral deflections (Figures 14a,c,e) and inter-story drift ratios (Figures 14b,d,f) under wind forces. Resulting in a total 3% increase of the total structural weight due to the insertion of the SBS, the additional structural system is proven effective in limiting the inter-story drifts. As can be seen in Figures 14b,d,f, the inter-story drift ratios before the insertion of the SBS are much greater than the maximum allowable value, especially at the upper modules, and they increase as the diagonal angle increases, due to the greater number of intra-module floors. From Figures 14a,c,e it is also evident that the insertion of the SBS does not affect the global stiffness of the building, since the top lateral deflection remains the same.

The efficacy of the SBSs has also been assessed in the investigation of real diagrid buildings, i.e. the Hearst Tower (New York) and The Bow (Calgary) [14]. Since the majority of diagrids are not stand-alone systems but present central cores that provide local floor-to-floor restraints to the diagonal members, avoiding their flexural engagement, the adoption of SBS-like structures can preserve the axial-dominated behavior in the diagrid structure, thus better exploiting the extraordinary efficiency of the external tube mechanism [14].

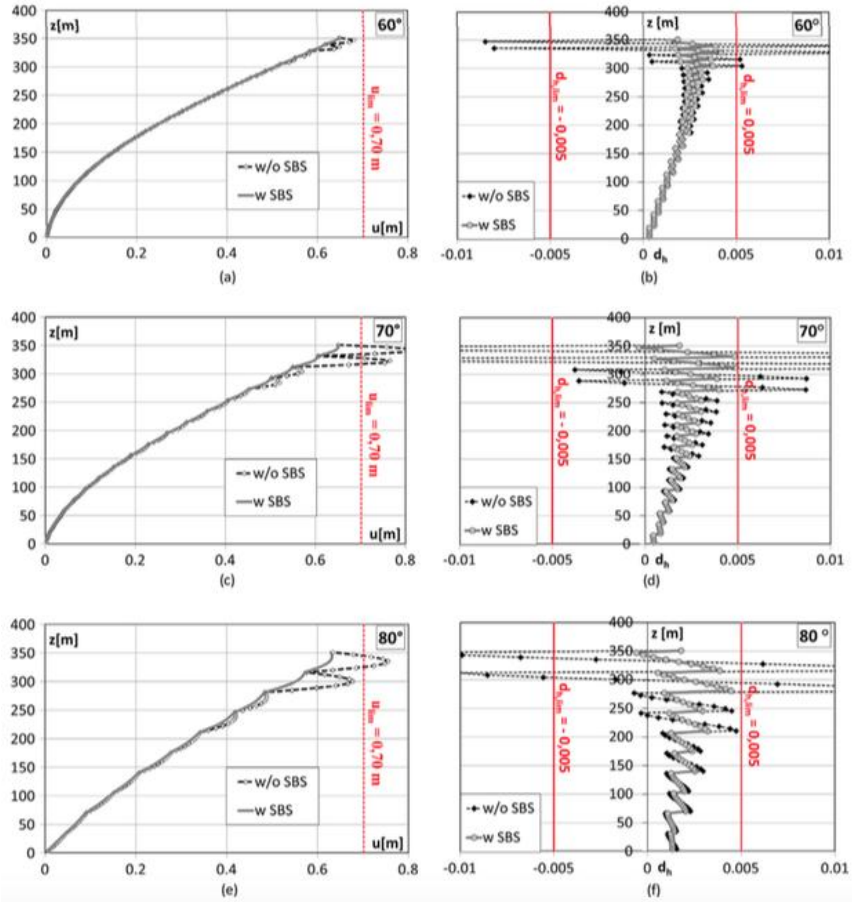


Figure 14. Efficacy of SBS in limiting inter-story drifts: (**a**-**b**) 60°; (**c**-**d**) 70°; (**e**-**f**) 80° diagonal pattern. (**a**,**c**,**e**) lateral deflections; (**b**,**d**,**f**) inter-story drift ratios. Used with permission from Montuori et al. [11].

The stability requirements for diagrid tubes are also investigated by Rahimian in [37]. SBSs are introduced in this paper, with suggestions regarding their design for column lateral bracing, similarly to the considerations of Montuori et al. [11]. However, in this analysis, the SBS aims also at stabilizing the diagrid itself, against the lateral buckling of the diagrid modules. The buckling deformation mode of the diagrid modules arises from the vertical loads acting on the diagrid nodes, both at the panel level and at the level of intermediate floors. The required stiffness of the SBS is function of the diagrid geometry and compression force in diagrid members. The SBS methodology proposed by the author is applied to the case of the Hearts Tower (New York), where the efficacy of the designed SBS is discussed. Besides the typical lateral deformations due to wind and seismic actions, limiting the lateral diagrid displacements due to buckling sway mechanisms under gravity loads is essential for an efficient structural behavior and design.

6. Shear-lag effect in diagrid tubes

One of the most important problems in external tubes composed of beams and conventional vertical columns is the shear-lag effect, which undermines their efficiency in high-rise buildings. As shown in Figure 15, for a framed tube subjected to later loads the actual axial force distribution in the vertical columns does not follow the Euler-Bernoulli distribution, i.e. linear and constant trend in the web and flange respectively [1]. Conversely, due to the nature of the framed tube with closely-spaced columns, both distributions are non-linear and result in higher stresses in the corner columns, compared to the ones in the middle of the flange. This phenomenon is known as the shear-lag effect. Shear-lag coefficients can be defined based on the non-linearity of the stress distribution in the web and flange façades. In the design of a framed tube, the limitation of the shear-lag effect often drives the design of the structural elements.

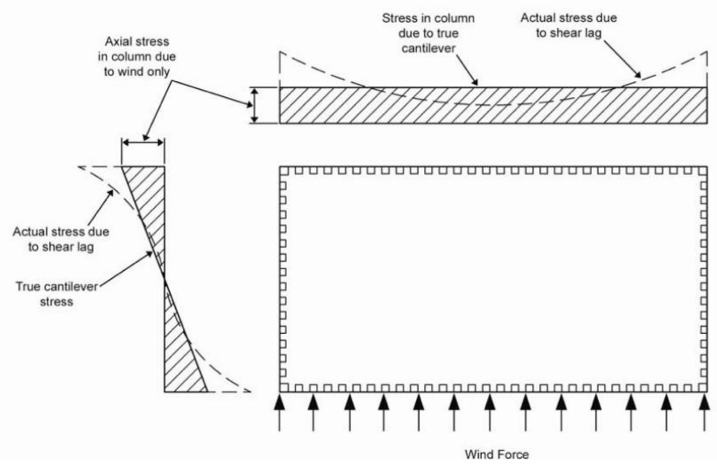
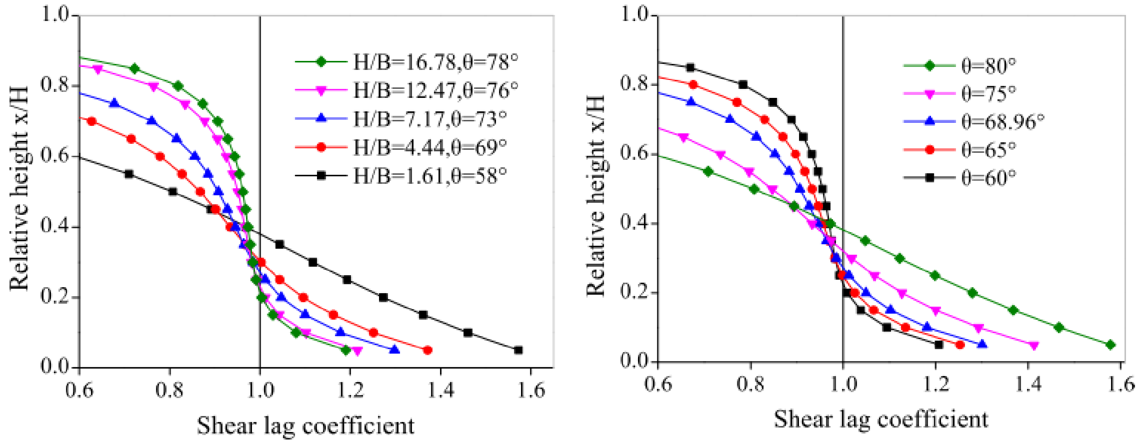


Figure 15. Shear-lag effect in framed tubes. Used with permission of Taylor & Francis Ltd (www.tandfonline.com), from Ali and Moon [1].

Diagrids are known to be stiffer than traditional framed tubes. However, being external tubular systems, they might experience the shear-lag phenomenon as well. For the first time, Leonard investigates the influence of the shear-lag in a 60-story tall square diagrid building, with comparison with the conventional framed tube solution [38]. It is found that the diagrid performs better both in terms of lateral deflection and shear-lag effect, compared to the framed tube. However, the shear-lag effect strongly depends on the external diagonal pattern. Steeper diagonal angles can increase the severity of the shear-lag effect, whereas the number of the diagonal bays on the building perimeter does not have a significant influence. Interestingly, it is also found that no direct correlation between the shear-lag and lateral deflection appears in diagrids. Sometimes, the solutions with higher shear-lag coefficients provide quite small lateral deflections [38]. This mainly derives from the different mechanical behavior of the diagrid with respect to the conventional framed tube: the former exploits the axial deformation of the external bracings, while the latter is dependent on the flexural and shear deformation of vertical columns and horizontal beams. As a consequence of the different mechanism, the shear-lag is less severe in diagrids than in conventional framed tubes.

The shear-lag effect is also analyzed for hybrid structures, where frame and diagrid tubes act together in different parts of the building [39]. In this study, it is still observed that the shear-lag effect in conventional framed tubes is much more significant than in diagrid systems. However, in hybrid diagrid-frame tubes, the shear-lag coefficients depend on the specific geometrical combination of the two systems over the height of the building and might not be negligible.

The previous studies investigate the shear-lag effects by means of FE calculations [38,39]. In a more recent work, Shi and Zhang [40] propose an analytical formulation for a quick evaluation of the shear-lag effect in diagrid tubes. To this purpose, the diagrid tube is equated to an elastic orthotropic membrane, where the material properties are derived based on the stiffness equivalence. Simple equations allow to compute the internal stresses in the equivalent tube under horizontal loads. From the non-linearity in the distributions of web and flange stresses, a measure of the severity of the shear-lag effect is provided. The suggested methodology is applied to a 52-story tall rectangular diagrid and validated against FE calculations. Different diagrid tubes, with different aspect ratios and diagonal angles, are also investigated. It is found that, for shorter buildings the shear-lag effect obviously increases; moreover, increasing the steepness of the external diagonals leads to greater shear-lag coefficients. Figure 16 shows the influence of aspect ratio (Figure 16a) and diagonal inclination (Figure 16b) on the maximum shear-lag coefficient along the height of the structure, as reported by Shi and Zhang [40]. The greater the distance of the shear-lag coefficient from 1, the greater the influence of the shear-lag effect. Therefore, in shorter diagrid buildings and/or with steeper diagonal inclinations, the shear-lag effect should be carefully taken into account in the design stages.



(**a**) (**b**)

Figure 16. Shear-lag coefficient in diagrids depending on the diagonal angle and building aspect ratio: (**a**) effect of aspect ratio under optimal angle; (**b**) effect of diagonal angle under fixed aspect ratio. Used with permission from Shi and Zhang [40].

7. Non-linear analyses and seismic performance of diagrid structures

Most of the studies that have been reported in Sections 2-6 deal with the investigation of the structural behavior of diagrid systems under static loads and within the linear elastic regime. Although linear static analyses can provide extremely important information for the preliminary design stages, the non-linear response of diagrid tubes is of paramount importance for the evaluation of their performance. In the same way, analyses under dynamic loading conditions can reveal significant information, especially regarding the seismic assessment as well as the resistance against progressive collapse.

One of the first works dealing with the seismic performance of diagrid tubes has been carried out by Kim and Lee in 2012 [41]. In the study, the authors analyze a set of 36-story tall square buildings, with diagonal angles ranging from 50.2° to 79.5°. Non-linear static analyses, i.e. push-over analyses, are carried out by applying lateral loads proportional to multi-mode story-wise distribution pattern. The non-linearity in the behavior of the structural members is also considered, according to FEMA-356 suggestions [42]. The outcomes show a quite brittle response for the diagrid structure, if compared to the traditional framed tube which shows more ductile behavior. Increasing the diagonal angle leads to lower ultimate shear forces carried by the diagrid before the final collapse. Non-linear dynamic analyses are also performed, where the equations of motion of the structure subjected to seven different earthquakes are numerically solved. The outcomes reveal that greater diagonal angles are usually found to induce greater lateral displacements. It is also found that both strength and ductility of the diagrid are increased when the diagonal members are replaced by buckling-restrained braces [41].

In a following paper, Kim and Kong [43] make use of non-linear static and dynamic analyses to investigate the resisting capacity of axisymmetric rotor-type diagrid buildings against progressive collapse. Based on arbitrary column removal scenarios, the robustness of 33-story tall diagrids, with cylindrical, concave, convex and gourd shapes, is evaluated. The outcomes show satisfactory resisting capability against progressive collapse when one or two diagonal members are removed from the first level, regardless of the geometrical shape. However, concave-type buildings exhibit lower collapse resistance when two pairs of bracings are removed from the first story. In the study, a thorough investigation of the collapse strength and formation of plastic hinges is also carried out, depending on the diagonal inclination and location of member removal [43].

The ultimate capacity of diagrids in the damaged state, when certain diagonals are removed from the nominal structure, is also investigated by means of FE non-linear analyses by Milana et al. [44]. The results show that the ultimate resistance of diagrids upon damaging is quite satisfactory, although it depends on the specific location of the bracing removal. In the same study, push-over analyses are carried out on a set of 40-story tall buildings, and their performance is evaluated in terms of strength, stiffness, ductility and sustainability aspects.

Although not common, experimental tests on prototype models can be also carried out to investigate the dynamic properties of diagrid tubes. For example, Liu et al. [45] conduce shaking table tests on a Plexiglas model of the Guangzhou West Tower, and compare the resulting dynamic features (mode shapes, vibrational frequencies, acceleration magnification coefficients, etc.) to FE time-history calculations. A crucial aspect in conducting such tests relies on the correct definition of the geometrical, inertia, stiffness and damping parameters of the prototype, which should reflect the real parameters of the tall building based on similarity laws. This procedure can also be a rational way to validate FE models [45].

The seismic assessment of diagrid towers has been further investigated in more recent papers [46–51]. In [46], Sadeghi and Rofooei quantify the seismic performance factors (SPFs) of steel diagrid buildings, i.e. the response modification coefficient (R-factor), the overstrength factor (Ω_0) and the displacement modification factor (C_d), based on FEMA P695 methodology [52]. FE push-over analyses and incremental dynamic analyses (IDAs) are carried out. It is found that diagrids exhibit a quite brittle behavior, as observed from the push-over curves, and the ductility increases as the diagonal angle increases. R-factors also depend on the diagonal inclination, varying between 1.5 and 3, for diagonal angles ranging from 45° to 71.5°. The restraining end-conditions of the diagonals (pin or rigid) do not have a significant influence on the stiffness of the structure; however, the pin-ended solutions better tolerate larger displacements, improving the building ultimate seismic performance.

In a series of following studies, Asadi et al. [47–49] perform a comprehensive investigation of the non-linear performance of low- to mid-rise steel diagrid structures, using static, time-history dynamic and incremental dynamic analyses. Special attention is paid to corner columns, due to the shear-lag effect, as well as to the diagonal inclination on the evaluation of the seismic assessment and loss estimation of very short (4- and 8-story tall) diagrid buildings [47]. Mid-rise buildings, in the 8- to 30-story range, are also investigated and their non-linear behavior is analyzed and compared to traditional solutions, such as moment resisting frames and concentrically braced frames, in terms of weight, story drift, lateral stiffness, fundamental period and evolution of plastic hinge formation [48]. A set of 4- to 30-story tall diagrid buildings is further investigated for the evaluation of the SPFs, and the authors recommend specific values of the SPFs for diagrid frames lying in different story ranges [49].

Very recently, Heshmati et al. [50] investigate the influence of the interior cores on the seismic performance of diagrid tubes. By the application of push-over analysis, it is found that the interior tube can indeed help as a backup load-resisting system after the yielding of the perimeter diagrid structure, procrastinating the insurgence of damage and providing an enhanced safety margin. Non-linear time-history analyses also reveal that most of the buildings perform well under severe earthquakes, dissipating large amount of the input energy and leading to quite uniform plastic hinges distribution [50]. The seismic reliability of diagrids is also recently investigated by Mohsenian et al. [51], who develop an efficient performance-based design strategy, based on a new multi-level response modification factor.

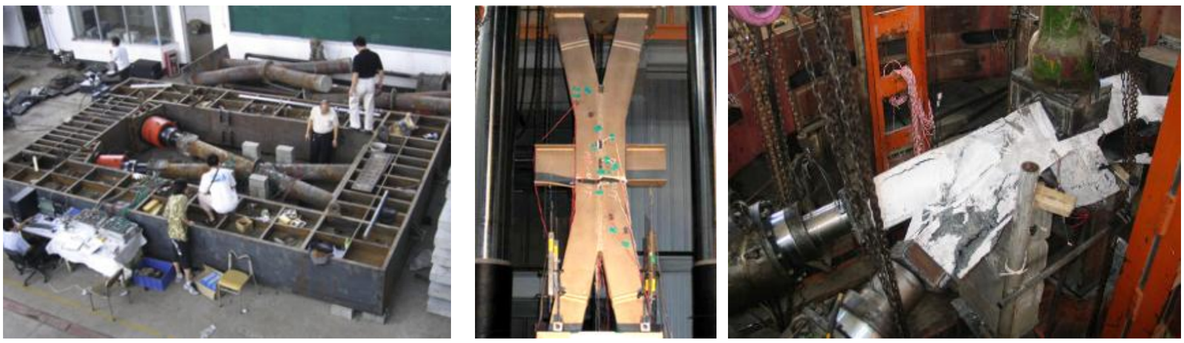
All these studies show that the non-linear and dynamic behavior of diagrids should be carefully taken into account right after the preliminary design stage, as it strongly depends on the diagrid features (diagonal angle, building height, etc.) that are often defined to satisfy the static requirements as shown in Section 4. As briefly remarked from the papers cited above, the analyses for the seismic assessment of structural systems can be very diverse. Various types of analyses can be carried out, such as linear modal analysis, non-linear static analysis (i.e. push-over analysis), time-history analysis, etc. These methodologies rely on the accurate modeling of the three-dimensional building and the non-linear behavior of the structural members needs to be properly taken into account for the correct comprehension of plastic hinge formation, local collapses, force redistribution, etc. The different modeling and design approaches can therefore lead to slightly different outcomes, that need to be related to the specific analysis and the adopted design approaches and modeling procedures.

8. Research on diagrid nodes

A crucial element in the assessment of diagrid structures lies in the correct design and realization of the nodes, where a connection usually consists of four intersecting diagonal columns and several beams (Figure 3a). These elements are pivotal as they are in charge of transferring the high axial stresses between the diagonal members. The failing of one single node induces a redistribution of the load path and might compromise the overall stability and resistance of the diagrid, especially under cyclic and dynamic loading conditions. As a matter of fact, due to the seismic concept "stronger connection, weaker component", special attention needs to be conveyed to the mechanical behavior of diagrid joints [53].

The joints are mainly divided in three types depending on the employed material, namely steel, reinforced concrete and concrete-filled steel tube (CFST) joints. Each of them is characterized by different mechanical behaviors, particularly under cyclic loading conditions and with reference to the hysteretic energy dissipation. Huang et al. [54] investigate the bearing capacity of CFST joints (Figure 17a), where the influence of connection detail, intersecting angle between the diagonals and loading type is analyzed on the bearing performance of the node. Based on the experimental results, it is found that the diagonal angle plays a key role in the definition of the joint failure mode, whereas the loading type (symmetric or asymmetric) has little influence. The authors also propose a simple equation for the calculation of joint bearing capacity, which is verified against the experimental outcomes. Kim et al. [55] perform an experimental campaign to analyze the cyclic behavior of the steel nodes from the Lotte Super Tower in Seoul (Figure 17b). Open- and box-section joints are realized and their cyclic performance is investigated in terms of stiffness and strength. Attention is also paid by the researchers to different welding methods. Subsequently, Jung et al. [56] study web-continuous steel connections for diagrid nodes under cyclic loads. Different welding methods and design details are taken into account, and they are not found to provide significant influence on the joint initial stiffness and yielding strength. Conversely, they can significantly modify the joint failure modes as well as the energy dissipation characteristics. Spatial concrete nodes are also studied by Zhou et al. [57], who investigate their failure mode and bearing capacity, focusing particular attention on the influence of transverse stirrups amount on the connection performance. It is shown that the volume ratio of transverse stirrups affects the bearing capacity of the joint, by effectively confining the concrete core under high compressive loads (Figure 17c).

These studies are fundamental for a thorough evaluation of the mechanical behavior of diagrid nodes, which in turn strongly affects the overall structural response of the diagrid system.



(**a**) (**b**) (**c**)

Figure 17. Research on diagrid nodes: (**a**) CFST joint, used with permission from Huang et al. [54]; (**b**) steel joint, used with permission from Kim et al. [55]; (**c**) concrete spatial joints, used with permission from Zhou et al. [57].

9. Twisted, tilted, tapered, freeform diagrids

Besides the notable structural efficiency of diagrid tubes in resisting lateral forces, one of the key factors which has led to their successful exploitation is the capability to realize complex-shaped structures. As a matter of fact, due to the versatility and modularity of the elementary triangular unit, diagrid systems can be effectively employed to build unconventional towers, such as twisted, titled, tapered and even freeform structures.

These unconventional shapes are deeply investigated by Moon in [58–60]. 60-story tall twisted diagrid towers are analyzed under lateral loads, with various twisting rates, namely 0, 1, 2 and 3 degrees per floor [58,59]. It is found that, as the rate of twist increases, the diagrid lateral stiffness decreases and the top lateral deflection increases. This is mainly due to the fact that, the reference un-twisted structure being designed with the optimal diagonal angle, increasing the twisting leads to higher deviation of the diagonal angle from its optimal value. This in turn causes the lower efficiency of the twisted tower compared to the un-twisted structure. The same analysis is conducted for 80- and 100-story tall diagrid buildings and the same conclusions are drawn in [59]. The performance of twisted diagrids is also investigated in terms of progressive collapse resistance and seismic performance by Kwon and Kim [61]. Based on arbitrary column removal scenarios on a set of 36-story tall twisted diagrid buildings, it is shown that the resistance against progressive collapse is decreased as the twisting angle increases, whereas the twisting angle is beneficial for improving the failure probability under seismic events.

Tilted towers are also investigated in [58,59], under both gravity and lateral loads. For 60-story tall diagrid buildings with various tilting angles (ranging from 0° to 13°), it is found that the top lateral displacement due to wind loads is not significantly affected by the tilting angle. Conversely, lateral displacements due to the eccentricity of gravity loads in tilted towers are found to be remarkably significant and these can become even greater than the lateral displacements due to horizontal actions for great tilting angles. This aspect obviously needs to be taken into account carefully for the realization of tilted diagrid structures.

Moon [59,60] also investigates the efficiency of tapered buildings compared to traditional vertical structures. Such an effectiveness arises from the more rational employment of the structural material in tapered tall buildings since this is more abundant in the lower part of the structure, where the gravity, shear and bending actions are more important. As pointed out by the author, attention should be paid when generating the tapered diagrid frame, as the inclination of the external façades obviously affects the inclination of the diagonal members, which is known to be a crucial parameter for the diagrid behavior. The analysis is carried out on a set of 60-, 80- and 100-story tall square diagrids under wind loads, with taper angles of 0, 1, 2 and 3 degrees. From the outcomes, it is shown that, as the taper angle increases, the top lateral deflection decreases, thus enhancing the lateral stiffness of the diagrid. This result is more significant as the building aspect ratio increases [59,60].

Finally, diagrid structures with irregular shapes along the building elevation, namely freeform diagrids, are also analyzed. In particular, in [59] freeform geometries are generated using "sine" curves of various amplitudes and frequencies. Lateral loads are applied to the freeform buildings and the outcomes show that the top lateral deflection increases as the freeform shape deviates more from the original rectangular box form.

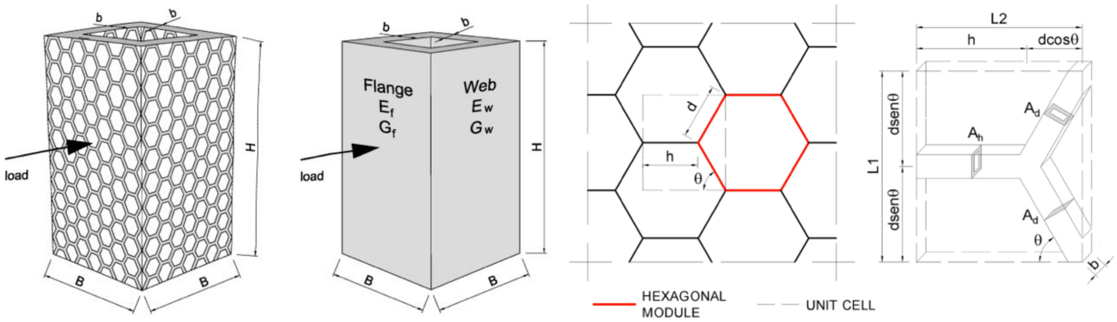
From the previous considerations, it is clear that twisted, tilted, tapered and freeform diagrid systems offer a great variety of architectural solutions to the design of unconventional tall buildings. However, their structural performance needs to be carefully evaluated from the early design stages, in order to lead to feasible and sustainable solutions.

10. New evolutions of grid structures: hexagrids and beyond

A further development of grid tubular structures for the realization of tall buildings has been inspired by natural materials, such as the honeycomb pattern (beehive). This is the case of hexagrids, where six-node hexagonal elements are placed all over the building perimeter to resist gravity and lateral loads. Examples are the Sinosteel International Plaza in Tianjin and the Al Bahr Towers in Abu Dhabi. Hexagrids can mainly be divided in two types, according to the orientation of the hexagonal cell: horizontal hexagrids, where the hexagon is composed of four diagonal members and two horizontal beams, and vertical hexagrids, where the four diagonals are coupled with two vertical columns. The different orientation of the basic hexagonal unit obviously leads to differences in the structural performance, which needs to be investigated in the preliminary design.

Although the concept of hexagrids is very similar to diagrids, both exploiting an external tubular structure to withstand external actions, their structural behavior is someway different. As a matter of fact, diagrids resist gravity, shear and bending actions mainly by the axial stress of the diagonal members. Conversely, besides the axial forces in the hexagonal members, the resisting mechanism of hexagrids also involves the bending deformation of the diagonals and of the horizontal/vertical elements.

Based on the seminal work from de Meijer [62], Montuori et al. [63] investigate the mechanical properties of hexagrid structures and their applicability in tall buildings. A general homogenization approach is applied, where the hexagrid tube is converted into an equivalent orthotropic solid membrane (Figure 18a). The hexagonal module and the unit cell are defined (Figure 18b) and representative volume elements (RVEs) are identified based on the loading conditions. A stiffness-based approach is then followed to calculate the equivalent elastic properties of the solid tube, based on the grid mechanical and geometrical properties. Note that in Figure 18 the horizontal hexagrid is shown, with diagonal members and horizontal beams. The same scheme has also been adopted to investigate hexagrids with vertical elements [63].



(**a**) (**b**)

Figure 18. (**a**) Analogy between the hexagrid tube and an orthotropic solid membrane; (**b**) hexagonal module and unit cell. Used with permission from Montuori et al. [63].

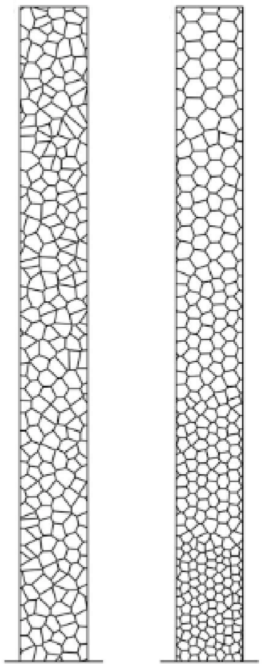
Based on the stiffness equivalence, both the equivalent elastic axial modulus E^* and shear modulus G^* are evaluated, and subsequently employed to calculate the displacements of the building equivalent cantilever. The effect of rigid floor diaphragms is also investigated, as it is found to have a strong impact on the evaluation of E^* through the modification of the RVE. The application of the simplified methodology is carried out on a 90-story tall hexagrid building, by changing the module height and the inclination of the diagonal members. A comparative analysis is also performed with similar diagrid structures [63]. From the results, it is found that the optimal angle of the diagonal members is close to 60° for horizontal hexagrids, whereas it is lower for vertical hexagrids, lying in the range 40°–50°. Compared to diagrid structures, the hexagrids are usually less stiff, being more bending-dominated, and consequently less structurally efficient. However, they can provide new architectural solutions with notable aesthetic effects.

The structural performance of hexagrid systems is also investigated by Lee and Kim [64] on 60-story tall square buildings. Different patterns of horizontal and vertical hexagrids, coupled with a central core, are analyzed under gravity and load actions. It is found that the vertical hexagrids are usually stiffer than the horizontal ones under lateral actions. This is mainly due to the axial contribution of the vertical elements in the flange façades. The gravity loads are equally distributed between the central core and the perimetral hexagrid, regardless the specific hexagonal pattern, whereas the lateral loads are absorbed by the hexagrids for the 50–80%, with differences depending on the arrangement of the hexagon module [64].

Hexagrids are also investigated under dynamic loading conditions such as earthquakes, as well as under arbitrary member removals to assess their resistance against progressive failure. In [65], Mashhadiali and Kheyroddin show that the shear mode deformation in hexagrids is usually greater than that occurring in diagrids, and hexagrids exhibit greater ductility under dynamic loadings. In a following paper, 28- and 48-story tall buildings with diagrid and hexagrid solutions are investigated in terms of resistance against progressive collapse, upon removal of corner elements [66]. The outcomes of non-linear static and dynamic analyses show that, although the specific geometrical configurations play an important role, hexagrids seem to be less vulnerable to progressive failure than diagrids, as they show greater potential for force redistribution.

Hexagrids are not the only further development of grid tubular systems in tall buildings. Taranath et al. [67] investigate the efficiency of hexagrids compared to another grid system, the pentagrid. The latter is based on the arrangement of various pentagons on the surface of the building, where all the elements are designed to share a similar amount of stress. From the outcomes of the structural analysis, the authors find that the pentagrid is more structurally efficient than the hexagrid, although the cost of constructability of the pentagrid might be superior [67]. Other grid evolutions also count octagrids and Voronoi-like grid systems.

Voronoi tessellation has been exploited in recent works as a new solution for grid tubular systems [68–70]. Angelucci and Mollaioli focus their attention on the evaluation of the mechanical characteristics of irregular Voronoi-like patterns for tall buildings [69]. Starting from a regular hexagrid solution, irregularity in the pattern is applied through random parametric generation, to realize more irregular building models (Figure 19a). The effect of varying-density pattern in the irregular Voronoi-like grid is also taken into account by the researchers (Figure 19b). Static and dynamic analyses are carried out on square 351-meter tall buildings. The outcomes reveal that cell irregularities do not affect the lateral stiffness significantly, and that the gradually rarefication of the pattern is a suitable strategy to optimize the lateral response [69].



(**a**) (**b**)

Figure 19. Irregular Voronoi-like grid pattern for tall buildings: (**a**) uniform density; (**b**) gradually rarefying density. Used with permission from Angelucci and Mollaioli [69].

The mathematical and numerical framework for the stiffness homogenization procedure of Voronoi-like grid tubes is thoroughly presented in [70], where the authors define the concept of the testing volume element (TVE), which replaces the RVE used in regular hexagrid structures. Based on numerical analyses, the polynomial expressions for the correction factors of the mechanical properties of the homogenized tube are proposed, when dealing with irregular Voronoi-like grids.

FE calculations on various 351-meter buildings are also carried out to investigate the efficiency of different Voronoi-like patterns [70].

Conclusions

In this paper, a fairly complete and up-to-date review of diagrid structural systems has been provided. The fundamental characteristics of diagrid tubes, which rely on the axial-dominated mechanism of the external mega-bracing, have been shown in the beginning of the paper, together with an overview of the structural solutions which brought to the realization and success of diagrids in recent years.

The simplified approaches for the preliminary design, based on the seminal works of Moon et al. [10] and Montuori et al. [13], have been reported and their application thoroughly analyzed. Some of the recent simplified methodologies for the structural analysis of diagrids, which do not rely on the common FE calculations, have also been described. The great variety of works regarding the optimization of the diagrid performance based on the geometrical characteristics has been discussed and their implications analyzed. Local structural issues, such as excessive inter-story drifts and stability problems of the interior gravity columns, have also been addressed. A discussion regarding the shear-lag effect in diagrid rectangular tubes has also been provided, based on the current research literature. Space has also been given to the non-linear and dynamic analyses which have been performed in the last decade to assess the seismic performance of diagrid systems, as well as their resistance against progressive collapse. A quick overview of the research about diagrid nodes has also been carried out, as well as the analysis on unconventional shapes for diagrids, such as twisted, tilted, tapered and freeform towers. Finally, some final remarks have been provided regarding the latest evolution of the tubular grid structures, e.g. hexagrids, pentagrid and irregular patterns based on Voronoi tessellation.

Throughout this review, we have seen that diagrids are efficient systems for tall buildings. Their efficiency mainly relies on the mechanism of the tubular system, coupled with the axial-dominated behavior of the basic triangular element. Because of the modularity characteristics and versatility of the reticulated surface, complex-shaped buildings can be realized with remarkable aesthetic potential. We have also seen that the power of diagrids also relies in the capability to further optimize their structural performance based on the geometrical features. This is a crucial point for sustainability purposes. With the need to limit material resources, while complying to safety and serviceability requirements, diagrid (and in general grid-based) tubes are the major candidates for the realization of the efficient, attractive and sustainable tall buildings of the future. Further researches dealing with all these aspects, following multi-criteria approaches [71] and involving different professional and academic figures and competences, will certainly lead diagrid structures to be more exploited worldwide in tall building design and construction.

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