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On the Influence of Collinear Surface Waves on Turbulence in Smooth-Bed Open-Channel Flows

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1 This work investigates how turbulence in open-channel flows is altered by the passage
 2 of collinear surface waves by using experimental data collected with laboratory tests in
 3 a large-scale flume facility, wherein waves followed a current. Flow velocity data were
 4 measured with a Laser Doppler Anemometer and used to compute profiles of mean
 5 velocity and Reynolds stresses, and pre-multiplied spectra. The velocity signal containing
 6 contributions from the mean flow, wave motion and turbulence was decomposed using
 7 the Empirical Mode Decomposition (EMD), which is considered a promising tool for
 8 the analysis of velocity time-series from complex flows. A novel outer-length scale h_0
 9 proposed which separates the flow into two regions depending on the competition between
 10 the vertical velocities associated with the wave motion and the turbulent velocities
 11 imposed by the current. This outer-length scale allows for the identification of a genuine
 12 overlap layer and an insightful scaling of turbulent statistics in the current-dominated
 13 flow region (i.e. $y/h_0 < 1$). As the wave contribution to the vertical velocity increases, the
 14 pre-multiplied spectra reveal two intriguing features: (i) in the current-dominated flow
 15 region, the very large-scale motions (VLSM) are progressively weakened but attached
 16 eddies are still present; and (ii) in the wave-dominated flow region (i.e. $y/h_0 > 1$), a
 17 new spectral signature associated with long turbulent structures (approximately 6 and
 18 25 times the flow depth h) appears. These longitudinal structures present in the wave-
 19 dominated flow region seem to share many features with Langmuir-type cells.

20 **Key words:**

21 1. Introduction

22 Many flows occurring in marine, coastal and estuarine environments result from the
 23 superposition of surface waves and currents, the latter often driven by either tidal forcing
 24 or other long-range hydraulic-head differences. Turbulence features that emerge from
 25 wave-current interaction (WCI) influence a variety of environmentally- and ecologically-
 26 relevant processes such as sediment transport (e.g. Madsen & Grant 1976; Dyer & Soulsby

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27 1988; Blondeaux 2001; Green & Coco 2014; Fagherazzi *et al.* 2015), microbiota dynamics
28 (Guasto *et al.* 2012), transport of nutrients and contaminants (De Souza Machado *et al.*
29 2016) and evolution of saltmarshes (Fagherazzi *et al.* 2012; Francalanci *et al.* 2013).
30 For what concerns engineering applications, wave-current turbulence plays a key role in
31 dictating the power-output, the mechanical loads and wake dynamics of hydro-kinetic
32 marine turbines (Gaurier *et al.* 2013; De Jesus Henriques *et al.* 2014; Noble *et al.* 2020)
33 and the scour around marine and coastal structures (Sumer *et al.* 2013; Sumer 2014).

34 While its relevance is not in dispute, the study of turbulence in wave-current flows
35 is still in its infancy. The majority of existing experimental works focus on the analysis
36 of mean velocity and shear stress profiles, due to their importance for the modelling of
37 sediment transport (Soulsby *et al.* 1993). Only sporadically, the attention has turned
38 to investigating the structure of turbulence, in a broader sense, which results from the
39 interaction between currents and either opposed (e.g. Kemp & Simons 1983; Klopman
40 1994; Umeyama 2005, 2009b; Yuan & Madsen 2015; Roy *et al.* 2018) or following waves
41 (e.g. Van Hoften & Karaki 1976; Kemp & Simons 1982; Klopman 1994; Umeyama 2005,
42 2009b; Carstensen *et al.* 2010; Yuan & Madsen 2015; Singh & Debnath 2016; Roy *et al.*
43 2017; Zhang & Simons 2019). All these studies agree on the fact that wave-current
44 interaction is strongly non-linear, namely that the mean flow properties of the combined
45 flow does not match those resulting from the linear superimposition of the current-alone
46 and wave-alone flows. For example, compared to current-alone flows, combined flows in
47 which waves follow a current display mean velocity higher near the bed and lower in the
48 upper part of the water column, and dampened Reynolds stresses (e.g. Umeyama 2005,
49 2009b; Singh & Debnath 2016).

50 However, there is no clear understanding of how and why different velocity statistics
51 respond to different combinations of waves and currents. Most experimental results are
52 presented dimensionally because there is no general agreement on the correct scaling
53 that should be employed to compare velocity statistics as measured in different flow
54 conditions. Further, the characterization of turbulence in terms of dominant eddies (i.e.
55 the eddies bearing the largest contribution to different turbulent kinetic energy compo-
56 nents) resulting from the non-linear interaction between waves and currents remains
57 largely unexplored. This knowledge-gap represents a bottleneck for the development of
58 appropriate and physically-based modelling strategies and it is not surprising that past
59 attempts to model combined wave-current flows obtained fair but limited success (see e.g.
60 Grant & Madsen 1979; Myrhaug 1984; Davies *et al.* 1988; Huang & Mei 2003; Olabarrieta
61 *et al.* 2010; Tambroni *et al.* 2015).

62 Much of the literature devoted to the study of combined wave-current flows at a
63 fundamental level relates to experimental studies carried out in laboratory settings. The
64 commonly employed approach involves exploring how the mean and turbulence flow
65 properties of a current-alone flow (i.e. the benchmark flow) are altered by the passage
66 of waves with different frequency and amplitude. In this respect, the present paper is
67 no different. However, with respect to past studies, it overcomes some experimental
68 shortcomings that are now presented and discussed to highlight some of the novelties
69 introduced herein.

70 Most previous laboratory studies were carried out by establishing flows with aspect
71 ratios (i.e. the ratio between the channel width and the flow depth) lower than five,
72 value that Nezu & Nakagawa (1993) indicated as the threshold below which lateral walls
73 affect turbulent properties in the mid cross-section of current-alone flows. For combined
74 wave-current flows, such lateral-wall effects have never been systematically investigated
75 and are largely unknown hence, when comparing combined with current-alone flows, low

76 aspect ratios make it difficult to discern whether the observed differences in turbulence
 77 properties are due to lateral walls' or waves' effects.

78 The aspect ratio is also known to significantly affect the scaling of energetic large eddies
 79 populating current-alone flows (often referred to as Very-Large Scale Motions, VLSMs,
 80 see [Peruzzi et al. 2020](#); [Zampiron et al. 2020](#)). In an attempt to shed light on the size and
 81 scaling of dominant eddies emerging from the interaction between waves and currents,
 82 this is an issue that should be taken into account when interpreting experimental data
 83 but, so far, it has been ignored probably because the interlinks between VLSMs and
 84 aspect ratio in open-channel flows have been identified only very recently.

85 Another shortcoming of past studies relates to the fact that benchmark flows (i.e.
 86 current-alone) were never established with boundary layers covering the entire water
 87 column. This, besides not being representative of flow conditions normally encountered in
 88 the field ([Sellar et al. 2018](#)), implies that waves were superimposed to "hybrid" shear flows
 89 displaying boundary layer properties up to some elevations from the bed and not well-
 90 defined (and difficult to replicate) features further above where, presumably, residual inlet
 91 turbulence persists. Such residual turbulence is facility-dependent and hence prevents
 92 experimental data to display flow features of general validity.

93 To advance the comprehension of turbulence in combined wave-current flows, the
 94 present study reports results obtained from novel experiments involving waves that follow
 95 a steady current generated in a laboratory smooth-bed open-channel flume. Turbulence
 96 statistics obtained from an unperturbed open-channel flow were used as benchmark to
 97 study the alterations caused by the passage of waves in combined wave-current flows
 98 involving a range of wave amplitudes and frequencies. The water surface level was
 99 monitored using five ultrasonic gauges positioned along the flume and the 2-D flow
 100 velocity field was measured using a Laser Doppler Anemometer (LDA). Much of the
 101 aforementioned experimental limitations are here overcome because: (i) the aspect ratio
 102 was kept above five to minimise lateral walls effects on turbulence statistics in the
 103 centreline of the flume where the measurements were collected; (ii) the benchmark (i.e.
 104 current-alone) experiment displayed a boundary layer thickness coinciding with the water
 105 depth and well defined turbulence properties as per self-similar turbulent open-channel
 106 flows over smooth beds; (iii) and VLSMs properties in the benchmark experiment were
 107 well documented and classified.

108 The experimental procedure and the employed laboratory equipment used to carry
 109 out the experiments are described in the next section (Section 2). Section 3 is then
 110 dedicated to the description of the signal-decomposition technique (the Empirical Mode
 111 Decomposition, EMD) that was employed to extract the turbulent signal from velocity
 112 measurements and hence to compute some of the velocity statistics used to interpret
 113 turbulence in combined wave-current flows. In Section 4 results are presented and
 114 discussed starting from the analysis of mean velocity profiles (Section 4.1) where we
 115 identify a novel length scale h_0 , which we prove key for the analysis and interpretation
 116 of turbulence in combined flows. This was explored through the analysis of second-
 117 order moments of velocity turbulent fluctuations (Section 4.2) and spectral analysis
 118 (Sections 4.3–4.4). The latter was successfully employed to investigate the fate of VLSMs
 119 in combined flows as well as to identify, for the first time, other large-scale structures
 120 that we speculate being induced by wave motion in ways that are somewhat similar to
 121 those responsible for the generation of Langmuir turbulence in ocean flows. In Section 5
 122 we present some discussions to interpret our results while Section 6 is finally devoted to
 123 some conclusions where we summarise the main results of the present paper.

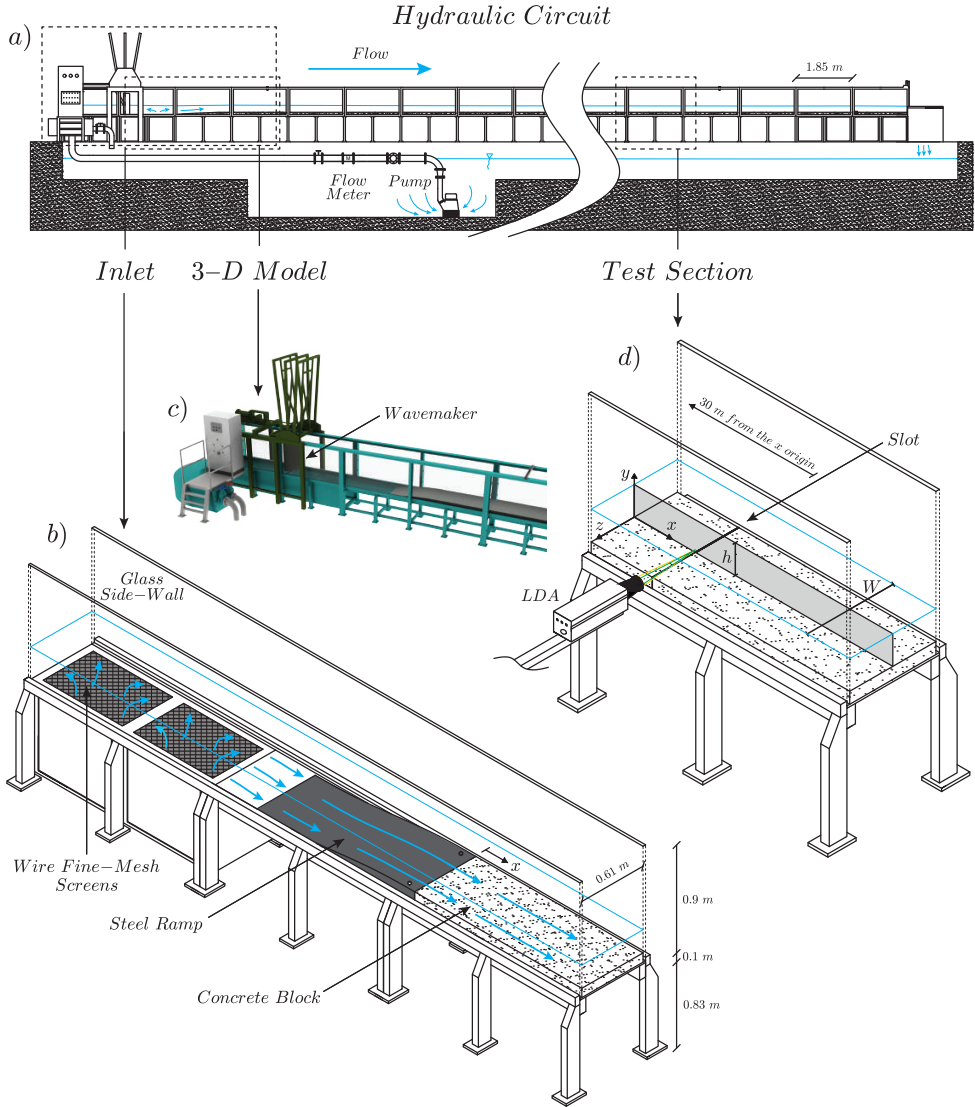


FIGURE 1. Overview of the flume: (a) sketch of the whole hydraulic circuit; (b) detail of the inlet configuration; (c) 3-D model of the inlet configuration and wavemaker; (d) detail of the test section. Panel (d) also shows the system of coordinate axes used in the present study (i.e. the longitudinal x , vertical y and spanwise z directions), the flow depth h and the channel width W . The origin of the longitudinal coordinate x is located at the downstream end of the steel ramp, as indicated in panel (b).

2. Methodology

2.1. Equipment

The experiments were carried out in the same flume facility and with the same setup and instrumentation as those described in [Peruzzi *et al.* \(2020\)](#). For this reason, in the text that follows we provide only a brief description of the equipment; for further details we encourage the reader to refer to [Peruzzi *et al.* \(2020\)](#).

The experiments were conducted in a non-tilting, recirculating open-channel flume at the Giorgio Bidone Hydraulics Laboratory of Politecnico di Torino (figure 1a). The flume

has glass sidewalls and is 50 m long with a rectangular cross-section 0.61 m wide and 1 m deep. To allow for near-wall LDA measurements (described below), the flume bottom was raised with smooth concrete blocks over the original bed. Close to the inlet section, the original bed and the concrete blocks were gently connected by a stainless-steel ramp (figure 1b–c), which was designed to prevent boundary layer separation (Bell & Mehta 1988) and hence the shedding of undesirable large scale eddies in the developing flow. To reduce the incoming turbulence generated by the hydraulic circuit, a series of wire fine-mesh screens were located in the sump underlying the flume inlet (figure 1b). For all the experiments, the test section was located at $x = 30$ m (the longitudinal, vertical and spanwise coordinates are indicated with x , y and z , respectively, and defined as indicated in figure 1d) from the origin (see figure 1b). As discussed in Peruzzi *et al.* (2020), at this distance, current-alone flows lose memory of inlet conditions and display self-similar vertical profiles of velocity statistics (as measured in the mid cross-section) that are in line with past literature on smooth-wall open-channel flows.

The flume used for the experiments allows for the generation of progressive surface waves by means of a piston-type wavemaker placed in proximity of the flume inlet (figure 1c). Three types of experiments were carried out involving, wave alone (WA), current-alone (CA) and combined wave-current (WC) flows. The channel outlet for the WA experiments was sealed with a steel cap downstream of a passive porous steel wave-absorber that absorbed about 91–94% of the wave total energy (estimated involving a simplified version of the two fixed probes method; Isaacson 1991) and hence prevented to a large extent wave reflections. The channel outlet for CA and WC flows was made of a rectangular sharp-crested weir, which was used to regulate the water depth h .

For all the experiments, water depths were measured with five ultrasonic gauges (sampling frequency f_s equal to 100 Hz) that were displaced along the flume, specifically at $x = 3.1$, 21.1, 27.1, 30.8 and 39.8 m, respectively. The nominal accuracy of the ultrasonic gauges is ± 1 mm and their performance in the measurement of the wave surface characteristics is comparable to that of classical instrumentations such as resistive or pressure sensors (Marino *et al.* 2018).

The near-wall LDA measurements were made by adopting the technique developed by Poggi *et al.* (2002) and subsequently used in other studies (Poggi *et al.* 2003; Escudier *et al.* 2009; Manes *et al.* 2011; Peruzzi *et al.* 2020). It consists in leaving a thin vertical slot (3 mm wide in this application) between two adjacent concrete blocks at the test section (figure 1d) so that the vertical laser beams can pass undisturbed and measurements near the wall can be taken with negligible alterations to the overlying flow (Peruzzi *et al.* 2020 reported that the effect of the slot on the flow was negligible). The 2-D LDA system used for the experiments is a Dantec Dynamics Flow Explorer DPSS working in backscatter configuration, the signal processing and acquisition were performed with two Dantec Dynamics Burst Spectrum Analyzers (BSA F600-2D) and a dedicated software (BSA Flow Software v6.5).

2.2. Experimental procedure and hydraulic conditions

2.2.1. Waves alone experiments

Prior to conducting experiments with waves following a current (WC), experiments with waves alone (WA) were carried out to study the characteristics of the waves generated with the adopted setup (figure 1b–c) and to determine the transfer function of the wavemaker, namely the relation between wave-amplitude and frequency imposed by the wavemaker and those of the waves actually propagating in the flume at various distances from the inlet. Table 1 reports the experimental hydraulic conditions for the WA

Run	h [cm]	f_w [Hz]	T [s]	a [cm]	H [cm]	L [cm]	$A_b \dagger$ [cm]	$U_w \dagger$ [cm/s]	h/L [-]	H/h [-]	ϵ [-]	U_R [-]
WA-T1	12.0	0.50	2.00	0.4	0.8	225.3	1.1	3.5	0.05	0.07	0.01	23.5
WA-T2	12.0	0.75	1.33	0.5	1.0	139.5	0.9	4.1	0.09	0.08	0.02	11.3
WA-T3	12.0	1.00	1.00	0.5	1.0	100.6	0.6	3.7	0.12	0.08	0.03	5.8
WA-T4	12.0	1.00	1.00	1.0	2.0	100.7	1.2	7.7	0.12	0.17	0.06	11.7
WA-T5	12.0	1.00	1.00	1.4	2.8	99.9	1.7	10.9	0.12	0.23	0.09	16.2

TABLE 1. Summary of the hydraulic conditions for the waves alone (WA) cases. The columns indicate: the mean water depth h ; the wave frequency f_w ; the wave period $T = 1/f_w$; the mean wave amplitude a ; the mean wave height $H = 2a$; the mean wave length L ; the longitudinal water particle semi-excursion due to the orbital motion at the bottom A_b ; the maximum longitudinal wave orbital velocity at the bottom U_w ; the relative depth h/L ; the relative height H/h ; the wave steepness $\epsilon = ak$, where k is the wave number $k = 2\pi/L$; and the Ursell number $U_R = HL^2/h^3$. Note that the symbol \dagger denotes values calculated by using the Airy linear wave theory (Dean & Dalrymple 1991).

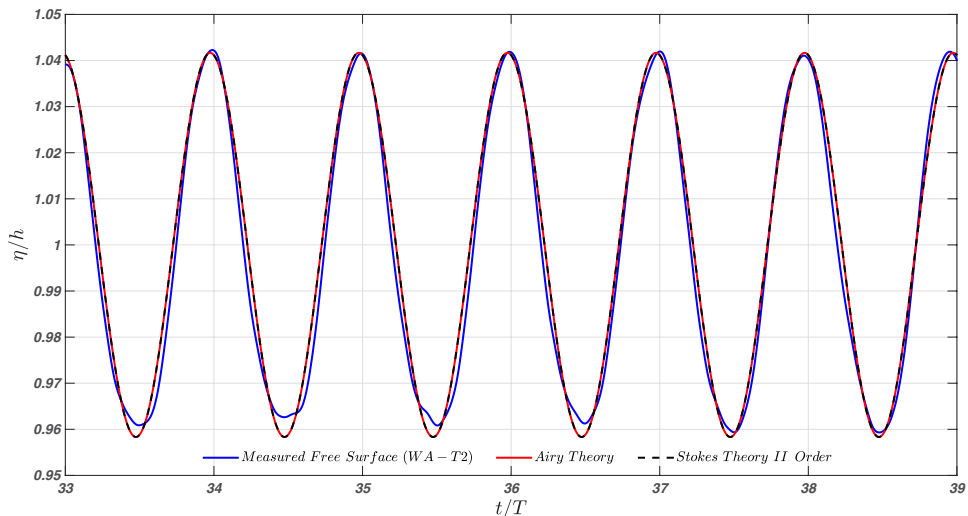


FIGURE 2. Temporal evolution of the normalized surface wave profile for the case WA-T2. The blue solid line represents the free-surface measured with the ultrasonic gauge in proximity to the LDA location ($x/h = 256$). The red solid and black dashed lines refer to the Airy linear theory and Stokes II order theory, respectively.

180 cases. The parameters h , a and T were determined from the water-surface measurements
 181 provided by the ultrasonic gauge placed in proximity to the LDA system (i.e. gauge
 182 number 4 at $x = 30.8$ m). The measurements lasted approximately 160 s so that it was
 183 possible to monitor 80–160 wave cycles, depending on the wave properties (table 1),
 184 with low reflection effects from the wave absorber placed at the channel end.

185 Based on the key wave parameters reported in table 1, it can be inferred that waves
 186 considered in the present study are in the intermediate water conditions and do not break
 187 ($0.05 < h/L < 0.5$, $\epsilon < 0.442$ and $H/h < 0.8$; Dean & Dalrymple 1991). According to
 188 Hedges (1995), the Airy or Stokes' II order wave theories are suitable to describe the
 189 waves generated in our experiments because both the Ursell number ($U_R = HL^2/h^3$)
 190 and the wave steepness have low values ($U_R \lesssim 40$ and $\epsilon \lesssim 0.125$). Focusing on the Airy

191 linear wave theory, the water particle longitudinal semi-excursion A_b and the maximum
 192 wave orbital longitudinal velocity U_w at the bed are defined as:

$$A_b = \frac{a}{\sinh(kh)} \quad (2.1)$$

$$U_w = \omega A_b \quad (2.2)$$

193 where $k = 2\pi/L$ is the wave number and $\omega = 2\pi/T$ is the wave angular frequency (Dean
 194 & Dalrymple 1991). Indeed, from the analysis of the temporal evolution of the free-surface
 195 profile η , reported in figure 2 for the representative test WA–T2, no substantial difference
 196 between the Airy (or Stokes’ II order) theory and the measurements is evidenced. The
 197 slight discrepancy in the wave troughs is approximately of the same order of magnitude
 198 as the ultrasonic gauge measurement uncertainty ($\pm\eta/h = 0.008$).

199 The wave attenuation along the flume was evaluated by comparing the wave heights
 200 measured by the ultrasonic gauges placed along the channel with the analytical results of
 201 Hunt’s wave attenuation theory (Hunt 1952). Even though the theory underestimates
 202 the wave attenuation, as already reported in previous studies (Grosch *et al.* 1960;
 203 Van Hoften & Karaki 1976), the general trend is well captured (not shown here). Overall,
 204 experimental data suggest that the waves generated in the flume facility (figure 1a) can
 205 be characterised with classical wave theories satisfactorily.

206 During the WA experiments, the wave-induced mass transport was not experimentally
 207 studied because it is extremely challenging to accurately quantify (Monismith 2020) and,
 208 above all, because the outlet boundary condition of the flume facility is different with
 209 respect to WC experiments (i.e. in the WA tests, the channel outlet was sealed with a steel
 210 cap and the wave-absorber is present, whereas in the WC test, the outlet was regulated
 211 with a tailgate and the wave-absorber was removed). This causes different return flow
 212 conditions between the two configurations, which make the comparison between the two
 213 sets of experiments very difficult.

214 2.2.2. *Combined wave-current experiments*

215 A comparative analysis of combined wave-current (WC) flows was carried out using
 216 a current-alone (CA) experiment as benchmark (see table 2). In WC experiments, the
 217 wave absorber was removed to prevent obstruction of the current outflow and both the
 218 pump and the wavemaker operated simultaneously. When the steady conditions for the
 219 CA case were set, the wavemaker was activated using the same input as for the WA
 220 cases (table 1) to generate the desired waves superimposed on the current. The hydraulic
 221 conditions for the WC cases are reported in table 2.

222 In the WC experiments the flow velocity was measured with the LDA in coincidence
 223 and non-coincidence mode. The former in order to have simultaneous longitudinal (u) and
 224 vertical (v) velocity measurements and therefore to estimate the Reynolds shear stress
 225 component, the latter to better resolve the turbulent spectrum at some elevations above
 226 the bed, as it allows for higher sampling frequencies of individual velocity components.
 227 In coincidence mode, the measurements were taken over 15 positions along the vertical
 228 coordinate for each run and 1000 wave cycles were measured at each position with a
 229 sampling frequency f_s ranging between 50 and 100 Hz. In non-coincidence mode, the
 230 velocity was measured at six selected positions for both the longitudinal and vertical
 231 components, with f_s of 150–300 Hz and sampling duration over 45 minutes. It is
 232 important to highlight that, due to the water surface level variation associated with
 233 the wave profile, the LDA velocity measurements were collected up to $y/h \approx 0.83$.

Run	h	u_τ	$U_b \ddagger$	f_w	a	H	$Re_b \ddagger$	$Re_\tau \ddagger$	$Fr \ddagger$	$RE \uparrow$	U_b/U_w	$a f_w/u_{\tau_c}$
	[cm]	[cm/s]	[cm/s]	[Hz]	[cm]	[cm]	[-]	[-]	[-]	[-]	[-]	[-]
CA	12.0	0.755	15.17	—	—	—	14400	1000	0.14	—	—	—
WC-T1	12.0	0.776	15.17	0.50	0.4	0.8	14400	1000	0.14	440	4.3	0.26
WC-T2	12.0	0.821	15.17	0.75	0.5	1.0	14400	1000	0.14	390	3.7	0.50
WC-T3	12.0	0.849	15.17	1.00	0.5	1.0	14400	1000	0.14	250	4.0	0.66
WC-T4	12.0	0.822	15.17	1.00	1.0	2.0	14400	1000	0.14	1050	2.0	1.32
WC-T5	12.0	0.794	15.17	1.00	1.4	2.8	14400	1000	0.14	2090	1.4	1.85

TABLE 2. Summary of the hydraulic conditions for the current-alone (CA) and waves following a current (WC) cases. The columns indicate: the mean water depth h ; the shear velocity u_τ ; the current bulk velocity U_b ; the wave frequency f_w ; the mean wave amplitude a ; the mean wave height H ; the current bulk Reynolds number $Re_b = R_h U_b / \nu$, where R_h is the hydraulic radius and ν is the kinematic viscosity of the water (equal to $0.907 \cdot 10^{-6}$ m²/s); the von Kármán number $Re_\tau = u_\tau h / \nu$; the Froude number $Fr = U_b / \sqrt{g h}$, where g is the gravitational acceleration; the wave Reynolds number $RE = A_b^2 \omega / \nu$, where $\omega = 2\pi f_w$ is the angular frequency; U_b / U_w is the ratio of current bulk velocity to longitudinal wave orbital velocity at the bottom and $a f_w / u_{\tau_c}$ is a parameter whose meaning will be better explained in the following. Note that the symbol \ddagger denotes values determined in the current-alone case (CA) and the symbol \uparrow denotes values determined in the waves alone case (WA).

234 Furthermore, 30-minutes long time-series of the free water surface were recorded by
 235 means of the ultrasonic gauges.

236 For all the experiments, the Froude number $Fr = U_b / \sqrt{g h}$ (where g is the gravitational
 237 acceleration) and the von Kármán number $Re_\tau = u_\tau h / \nu$ of the current were 0.14 and
 238 1000, respectively. The aspect ratio W/h was equal to 5.08 so that flow conditions at
 239 the mid cross-section of the channel could be considered unaffected by lateral walls
 240 (Nezu & Nakagawa 1993). The shear velocities u_τ reported in table 2 include the shear
 241 velocity for the CA case (u_{τ_c} ; for more details see Peruzzi *et al.* 2020) and the shear
 242 velocities for the WC cases ($u_{\tau_{wc}}$); both were estimated using the classical Clauser method
 243 (Clauser 1956), assuming the occurrence of a logarithmic layer in the near-wall region
 244 (details on the existence of a logarithmic layer will be found in Section 4.1) and using
 245 a von Kármán coefficient $\kappa = 0.41$ and constant $B = 5.5$ (values found for the CA
 246 case). The values of $u_{\tau_{wc}}$ are slightly higher than those of u_{τ_c} and this agrees with
 247 the detected increase in the gradient of the time-averaged free surface height, $S_w =$
 248 dh/dx , in the presence of waves. Indeed, the free surface slope S_w between the two
 249 ultrasonic gauges (i.e. gauges 3 and 4) adjacent to the LDA system is higher for the
 250 WC cases (S_w ranging from $-0.954 \cdot 10^{-4}$ to $-1.361 \cdot 10^{-4}$) compared to the CA case
 251 ($S_w = -0.815 \cdot 10^{-4}$). This seems reasonable because an increase in the shear velocity
 252 values in waves plus current experiments was already reported in the literature (Kemp
 253 & Simons 1982; Zhang & Simons 2019).

254 Based on the values of the current Reynolds number Re_b , the wave Reynolds number
 255 RE and the ratio U_b / U_w (table 2), the resulting combined boundary layers were turbulent
 256 for all the cases investigated (Lodahl *et al.* 1998), even though the wave boundary layers
 257 for the WA cases were laminar or transitional (Blondeaux 1987).

258 It should be noted that the difference in the mean values of the wave heights H
 259 between the WA and WC experiments, reported in table 1 and table 2, is almost
 260 negligible. However, experimental data obtained from the ultrasonic gauges indicate
 261 that WC experiments are somehow affected by the presence of the current. To evaluate
 262 these effects, figure 3(a–b) reports the coefficients of variation of the wave period

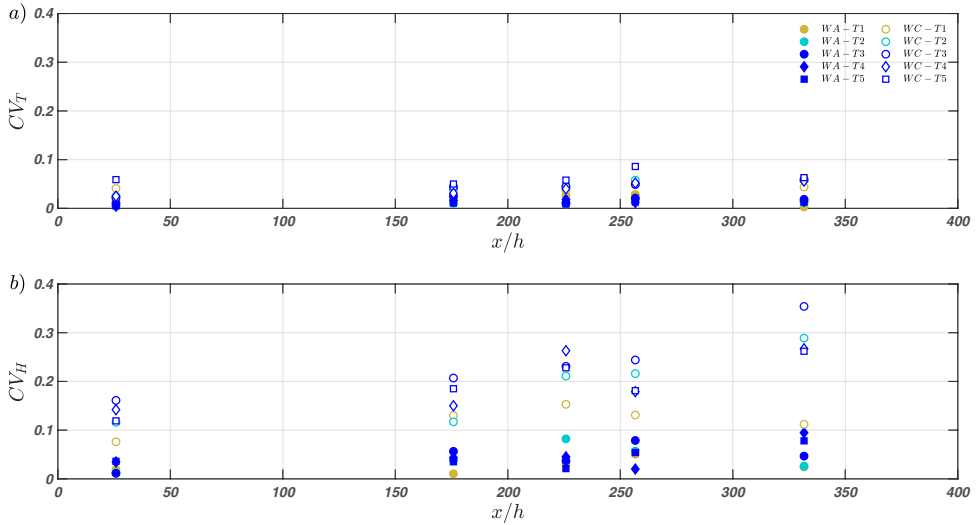


FIGURE 3. Coefficients of variation of (a) the wave period, and (b) the wave height for the WA (filled markers) and WC (hollow markers) experiments.

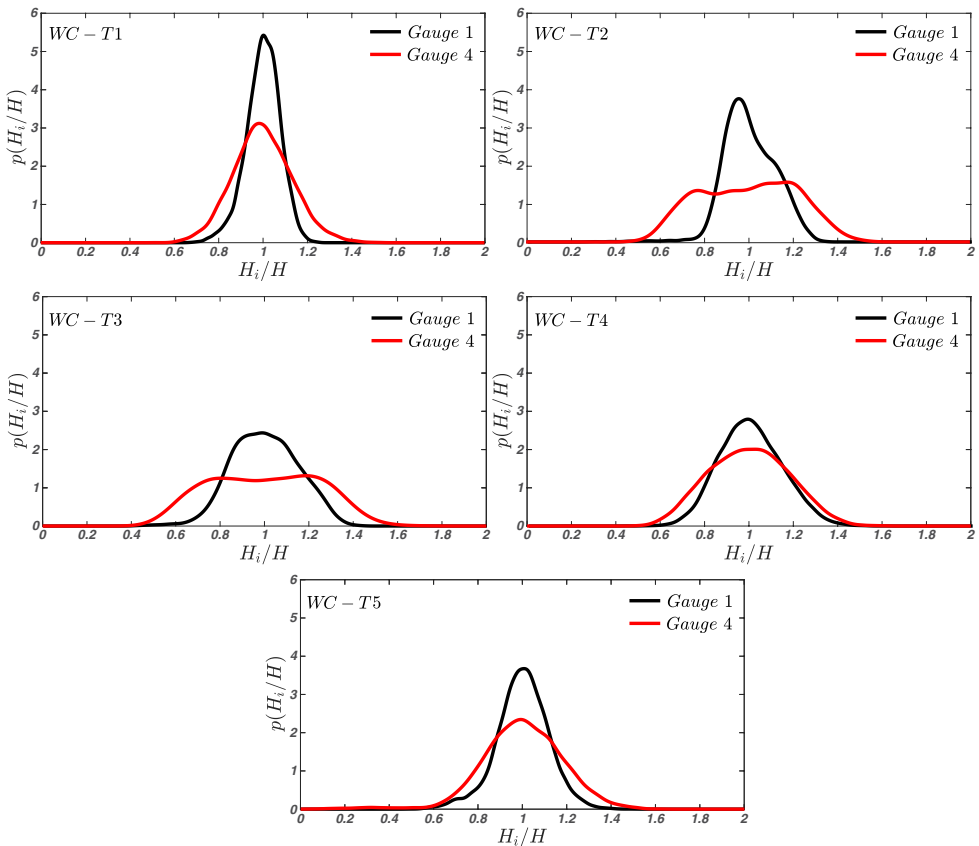


FIGURE 4. Data-estimated probability density functions (PDF) of wave heights for all the WC experiments recorded at gauge 1 (3.1 m from the origin, black) and gauge 4 (30.8 m from the origin, red).

263 $CV_T = T_{std}/T$ and wave height $CV_H = H_{std}/H$, where T_{std} and H_{std} are the wave
 264 period and height standard deviations while T and H are the mean values (table 1 and
 265 table 2), recorded at each ultrasonic gauge along the flume. While the values of CV_T
 266 are bounded between 0 and 0.1 for all the experiments with no obvious trend, indicating
 267 low variability around the mean, the values of CV_H for the WA and WC experiments
 268 display a different behaviour: the former show a negligible variation along the channel
 269 ($0 < CV_H < 0.1$), while the latter are spread across a wider range and show an increase as
 270 the waves move along the channel. A considerable increase in variability associated with
 271 the presence of the current is evident when comparing the same case with and without
 272 current (figure 3b).

273 To further characterise the variability of the wave heights in the WC experiments,
 274 figure 4 displays the probability density functions (PDF) of the wave heights estimated
 275 for each run at the gauges close to the flume inlet (gauge 1) and to the LDA system
 276 (gauge 4), respectively. The PDFs were computed directly from the data by using a
 277 non-parametric kernel distribution, that is often used with raw dataset in order to avoid
 278 making assumptions about the data distribution. In figure 4, the PDF is indicated as
 279 $p(H_i/H)$, where H_i is the i -th measured wave height, H is the mean wave height (table 2).
 280 Moving from the first to the fourth gauge, all cases show a flattening of the distribution
 281 that is particularly marked in WC-T2 and WC-T3.

282 This important alteration of the wave surface characteristics in the WC experiments
 283 was likely caused by multiple mechanisms, which require to be briefly discussed. Figure
 284 3(b) indicates that, with respect to the WA case, WC experiments display increased
 285 wave irregularity since the beginning of the flume. This suggests that, as observed by
 286 [Robinson et al. \(2015\)](#), the upwelling configuration of the inlet might induce free surface
 287 perturbations, which affect the generation of regular waves. More interestingly, figure 3(b)
 288 and figure 4 also show that, for all the experiments but mostly for WC conditions,
 289 waves' irregularity increases with increasing longitudinal distance from the inlet. Such
 290 an increase in WA experiments (for deep and intermediate waters) is likely to be caused
 291 by mechanisms akin to Benjamin-Feir instabilities ([Benjamin & Feir 1967](#)), which have
 292 been experimentally documented since [Benjamin \(1967\)](#). It is therefore likely that a
 293 similar instability mechanism makes the waves more irregular as they travel along the
 294 flume also in the WC experiments. However, the reason why a current could exacerbate
 295 such irregularity with respect to the WA experiments (see figure 3b) is not clear and is
 296 not further commented herein as it requires a dedicated study, which goes beyond the
 297 scope of the present paper. It is important to point out, though, that due to the observed
 298 non-uniform distribution of waves' characteristics along x , the investigated flows cannot,
 299 strictly speaking, be considered as "equilibrium (i.e. self-similar) boundary layers" (note
 300 that the CA alone experiment was identified by [Peruzzi et al. \(2020\)](#) to be in equilibrium
 301 to a very good approximation, so the source of non equilibrium can only come from
 302 waves' evolution along the flume). This means that at each location along the flume, it is
 303 not clear whether the WC boundary layers are either fully developed or not. However, to
 304 the authors' opinion, in combined wave-current flows this difficulty has to be embraced
 305 mainly because it is experimentally very challenging to generate well developed turbulent
 306 currents over distances that are short enough to consider wave properties reasonably
 307 uniform. Moreover (but this is a weaker justification) irregular and developing waves
 308 are the rule rather than the exception in the field ([Draycott et al. 2019](#)). Despite the
 309 non-uniform conditions and wave variability reported, we believe that the data analysis
 310 and interpretation reported herein lead to results that are fairly robust and supported
 311 by sound physical arguments.

312 Besides dealing with non-equilibrium conditions, the interpretation of experimental

313 results is made difficult by waves' irregularity, which makes it challenging to isolate the
 314 turbulence component of the signal and hence infer turbulence properties and structure.
 315 This problem is dealt with in the next section.

316 **3. Signal decomposition**

317 One of the challenges of studying turbulence in combined wave-current flows is the need
 318 for extracting and separating the turbulent and wave components of the raw velocity
 319 signal. Unsteady turbulent velocity signals can be decomposed according to the so-
 320 called triple decomposition (Hussain & Reynolds 1970). For instance, the longitudinal
 321 instantaneous velocity component can be decomposed as:

$$u = U + \tilde{u} + u' \quad (3.1)$$

322 where U is the time-averaged velocity, \tilde{u} is the periodic component (e.g. the periodicity
 323 imposed by the passage of waves) and u' is the turbulent component. The periodic
 324 component \tilde{u} can be determined with $\tilde{u} = \langle u \rangle - U$, where $\langle u \rangle$ is the phase-averaged
 325 velocity determined by averaging over an ensemble of samples taken at a fixed phase in
 326 the imposed oscillation and it is expressed as:

$$\langle u \rangle = \frac{1}{N} \sum_{i=1}^N u(t + iT) \quad (3.2)$$

327 where T is the period of the oscillation and N is the total number of cycles.

328 This signal analysis procedure is referred to as the phase-averaging method (Franca &
 329 Brocchini 2015) and is the most common employed technique in the study of combined
 330 wave-current flows (Kemp & Simons 1982; Umeyama 2005, 2009a; Singh & Debnath
 331 2016; Roy *et al.* 2017; Zhang & Simons 2019).

332 This technique is very sensitive to the regularity of the waves and if the waves
 333 are not perfectly monochromatic or do not present a repetitive pattern over time,
 334 it becomes very difficult to obtain reliable estimates of conditional statistics because
 335 there are mutual leakages between the wave and turbulent component of the signal.
 336 An alternative two-point measurement technique for separating turbulent and wave
 337 components was developed by Shaw & Trowbridge (2001). This technique utilises the
 338 velocity signals collected simultaneously by two sensors spatially separated so that the
 339 correlation between the two signals is associated with the wave motion only; namely,
 340 the sensors are located at a distance much larger than the turbulence integral scale, but
 341 much smaller than the wavelength of the surface waves (Hackett *et al.* 2011; Nayak *et al.*
 342 2015). This latter technique is not affected by irregular waves but it requires two-point
 343 measurements that are often available in laboratory settings but rarely in the field. This
 344 makes direct comparison of results difficult, due to the lack of a common protocol in
 345 data analysis procedures. Note that, to the authors' opinion, within the context of wave-
 346 turbulence interaction, results will be always partially dependent on the chosen signal
 347 decomposition technique so working on common grounds, namely widely accepted data
 348 analysis techniques, would be desirable in future studies.

349 In light of the limitations of the phase-averaging method in dealing with not perfectly
 350 monochromatic waves (see Section 2.2), in the current study we separated the turbulent
 351 and wave components employing the so-called Empirical Mode Decomposition (EMD).
 352 This technique was chosen because, besides working well for irregular signals resulting

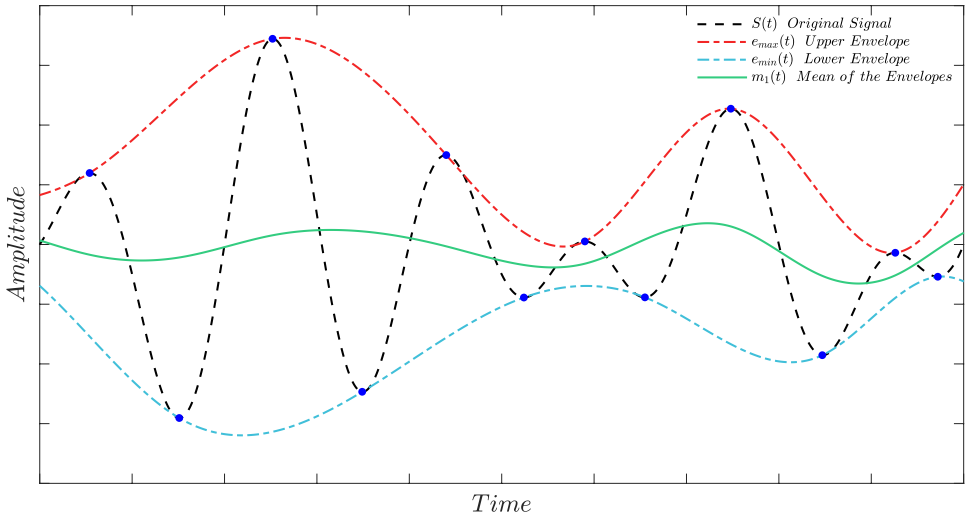


FIGURE 5. Identification of the signal extrema (blue dots), construction of the upper (red) and lower (blue) envelopes and computation of the mean envelope (green).

353 from non-linear interaction processes (such as wave-turbulence interactions), it does not
 354 require simultaneous multipoint measurements.

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3.1. Empirical mode decomposition

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The Empirical Mode Decomposition (EMD) was firstly proposed by Huang *et al.* (1998, 1999, 2003) for the analysis of non-stationary time series and has been used in numerous fields since then. Some successful applications in fluid mechanics are: the analysis of turbulent scales in fully developed homogeneous turbulence (Huang *et al.* 2008, 2010), the quantification of the amplitude modulation effects in wall turbulence (Dogan *et al.* 2019) and the study of wave-turbulence properties in the surf zone (Schmitt *et al.* 2009) or ocean-surface (Qiao *et al.* 2016).

Differently from most other methods (e.g. spectrogram or wavelet), the basic functions of the EMD are directly inferred from the data themselves and no signal features are assumed *a priori*. The main drawback of the EMD is that it is fully empirical and no rigorous mathematical foundations have been yet derived, although some theoretical justifications have been proposed (see, Flandrin *et al.* 2004). Nevertheless, the EMD procedure satisfies the perfect reconstruction property, namely the original signal can be reconstructed completely by summing all the functions that have been inferred from it. Such functions are referred to as Intrinsic Mode Functions (IMFs) and represent the natural oscillatory modes that are embedded in the signal. Any IMF must satisfy two conditions: (i) “in the whole dataset, the number of extrema (maxima and minima) and the number of zero-crossings must either be equal or differ at most by one”; and (ii) “at any point, the mean value of the envelope defined by the local maxima and the envelope defined by the local minima is zero” (Huang *et al.* 1998, 1999). Hence, the IMF represents an ideal zero-mean amplitude and frequency modulation function.

The IMFs are extracted from the signal by means of the so-called sifting process (Huang *et al.* 1998, 1999, 2003), which has two main purposes: (i) to eliminate riding waves, i.e. the presence of a local minimum (maximum) greater (lesser) than zero between two successive local maxima (minima); and (ii) to make the oscillatory profiles more symmetric with respect to zero.

382 The first step of the sifting process is the localization of the maxima and minima in the
 383 original signal $S(t)$. Then, the upper envelope $e_{max}(t)$ and the lower envelope $e_{min}(t)$ are
 384 reconstructed and the mean envelope can be calculated as $m_1(t) = (e_{max}(t) + e_{min}(t))/2$
 385 (figure 5). The envelopes are reconstructed by means of an interpolating function. The
 386 function involved in the interpolation of the maxima/minima varies according to the
 387 changes and improvements to the algorithm proposed by various authors (Lei *et al.* 2013),
 388 nevertheless the most used is the cubic spline interpolator. At this point, the function
 389 generated by the first round of sifting of the signal is determined as $h_1(t) = S(t) - m_1(t)$.
 390 However, $h_1(t)$ is rarely a true IMF and must be further processed to eliminate any riding
 391 waves until it respects the two IMF conditions. Therefore, the generated $h_1(t)$ is set as
 392 the new input time-series and the sifting process is repeated j times until the first IMF
 393 from $h_{1j}(t) = h_{1(j-1)}(t) - m_{1j}(t)$ is obtained. From the first IMF $C_1(t) = h_{1j}(t)$, the
 394 first residual is obtained by subtraction from the original signal, i.e. $r_1(t) = S(t) - C_1(t)$.
 395 If the residual $r_1(t)$ is either a constant, a monotonic function or a function with at
 396 most one local extreme point, the sifting process ends, otherwise $r_1(t)$ is used as the new
 397 input signal and the sifting is repeated from the first step. When no more IMFs can
 398 be extracted, the sifting ends with $(n - 1)$ IMFs and a residual $r_n(t)$. At this point the
 399 original signal $S(t)$ can be expressed as:

$$S(t) = \sum_{i=1}^{n-1} C_i(t) + r_n(t) \quad (3.3)$$

400 where $C_i(t)$ is the i th IMF following the order of extraction from the signal. Due to the
 401 nature of the EMD, $C_1(t)$ is the IMF with the highest characteristic frequency oscillation,
 402 while $C_{n-1}(t)$ has the lowest.

403 If too many sifting iterations are performed, the IMF reduces to a constant-amplitude
 404 frequency-modulated function, annihilating the intrinsic amplitude variations and mak-
 405 ing the results physically meaningless (Huang *et al.* 2003). To prevent this, the sifting
 406 iterations must be limited by means of a stopping criterion (e.g. Huang *et al.* 1998;
 407 Rato *et al.* 2008; Tabrizi *et al.* 2014). The sifting stopping criterion we employed is the
 408 Resolution Factor (Rato *et al.* 2008), which is based on the ratio between the energy
 409 of the original signal $S(t)$ and the energy of the average of envelopes $m_i(t)$ at the i th
 410 iteration, i.e.:

$$RF = 10 \log_{10} \left(\frac{S(t)^2}{m_i(t)^2} \right) \quad (3.4)$$

411 In particular, we used a threshold value of 45 dB as recommended by Rato *et al.* (2008).

3.2. Adopted procedure

413 In this work we implemented the EMD algorithm proposed by Rato *et al.* (2008),
 414 who improved the original procedure introduced by Huang *et al.* (1998) to minimise
 415 the impact of sensitive factors, such as: the extrema localisation, the method used to
 416 interpolate the extrema and calculate the envelopes, the handling of the end-points at
 417 the boundaries and the decomposition stopping criterion. The following procedure was
 418 adopted to separate the periodic (wave) and turbulent components of the original signal
 419 obtained from the WC experiments:

- 420 **Step I** obtain the IMFs and the residual from the signal by using the EMD algorithm;
- 421 **Step II** compute the spectrum of the IMFs;

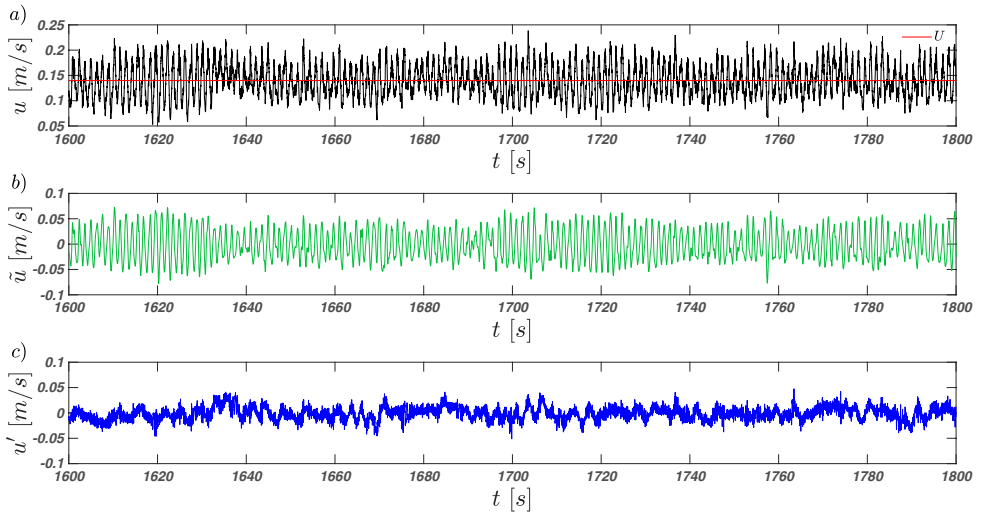


FIGURE 6. (a) Original signal (black) and mean velocity (red); (b) wave component (green) and (c) turbulent component (blue). Test WC-T2, $y/h = 0.1$, longitudinal velocity component.

422 **Step III** identify the IMFs that contain the wave signal based on the shape of the
 423 spectrum (i.e. the dominant peak/peaks associated with the wave motion);

424 **Step IV** obtain the wave component by summing up all the IMFs that contain the wave
 425 signal, the remaining components are summed up to obtain the turbulent component.
 426 This way the original signal is decomposed into wave and turbulent component;

427 **Step V** perform a visual check of the wave and turbulent components against the original
 428 signal to qualitatively assess whether all the wave oscillations have been separated from
 429 the signal. The features considered are the difference between the amplitude of the wave
 430 motion and the turbulent fluctuations together with wave-shaped patterns having the
 431 wave frequency inside the turbulent component;

432 **Step VI** if the quality check shows that some wave oscillations are still present in the
 433 turbulent component, then additional IMFs must be classified as wave components and
 434 handled accordingly. This step must be repeated until the turbulent component shows
 435 no obvious periodicity.

436 At the end of the process, the original signal (figure 6a) is decomposed in the wave
 437 (figure 6b) and turbulent (figure 6c) components. In the current study, for all the exper-
 438 imental conditions, the wave component was entirely embedded in 2–5 well-recognisable
 439 IMFs at most.

440 As clearly visible in the example displayed in figure 7, the adopted procedure creates
 441 an artificial valley in the power spectral density of the turbulent signal whose physical
 442 meaning is questionable. This happens because part of the turbulent energy with fre-
 443 quency bandwidth around the frequency of the wave motion results to be associated
 444 with the wave component instead of the turbulent component, creating a sort of spectral
 445 loss. Despite numerous attempts, we could not find any tuning of the EMD procedure
 446 that allowed for the removal of this valley and the associated loss. Therefore, it was
 447 decided to quantify its effects using a standardised procedure as follows.

448 Similarly to what was done by [Banerjee *et al.* \(2015\)](#) and [Vettori \(2016\)](#), this spectral
 449 loss was quantified as the area bounded between a power law (line of constant slope in
 450 log-log coordinates) and the artificial valley. The edges of the valley were chosen as the
 451 last/first spectral point after/before which an evident change in the trend identified by

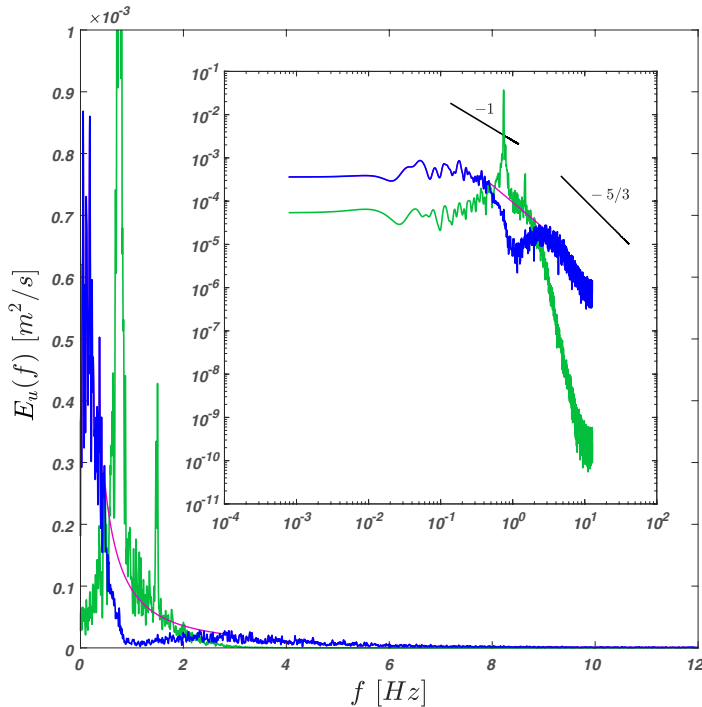


FIGURE 7. Spectra of the longitudinal velocity measured at $y/h = 0.03$ for the test WC-T2. Blue and green lines indicate the turbulent and wave component. The magenta line shows an example of how the artificial valley in the turbulent signal spectrum was bridged. The main and sub-plot show spectra in linear and log-scale, respectively. The straight black lines in the sub-plot represent power laws with exponents -1 and $-5/3$.

452 the previous/following ten spectral estimates was detected. Following this method, the
 453 loss resulted to be as 20%–30% of the total spectral energy for the longitudinal velocity
 454 and 10%–20% for the vertical velocity. Note that, after a careful sensitivity analysis, this
 455 estimates resulted to be weakly dependent on the exact location of the aforementioned
 456 edges of the power law, which we realise, is identified with a level of arbitrariness. Equally
 457 arbitrary is the choice of using a power law because the exact shape of the spectra in
 458 proximity of the valley is unknown. Despite these obvious shortcomings, the analysis
 459 above revealed that the relative magnitude of the spectral loss is roughly constant and
 460 independent of flow conditions. This indicates that the turbulent velocity variances are
 461 probably underestimated by the EMD procedure (note that $\sigma_v^2 = \int E_u(f) df$, e.g. Bendat
 462 & Piersol 2011), however their behaviour in response to different wave forcing (i.e. the
 463 response in terms of trends instead of actual values) is likely to be preserved and captured.
 464 Finally, it is worth noting that the spectral analysis presented in Sections 4.3–4.4 was
 465 conducted on the complete velocity signal to avoid potential impact on the estimated
 466 scales of VLSMs.

467 4. Results

468 4.1. Mean velocity profiles

469 The vertical profiles of the time-averaged longitudinal velocity for the waves following
 470 a current (WC) and current-alone (CA) cases (table 2) are reported in figure 8(a). With

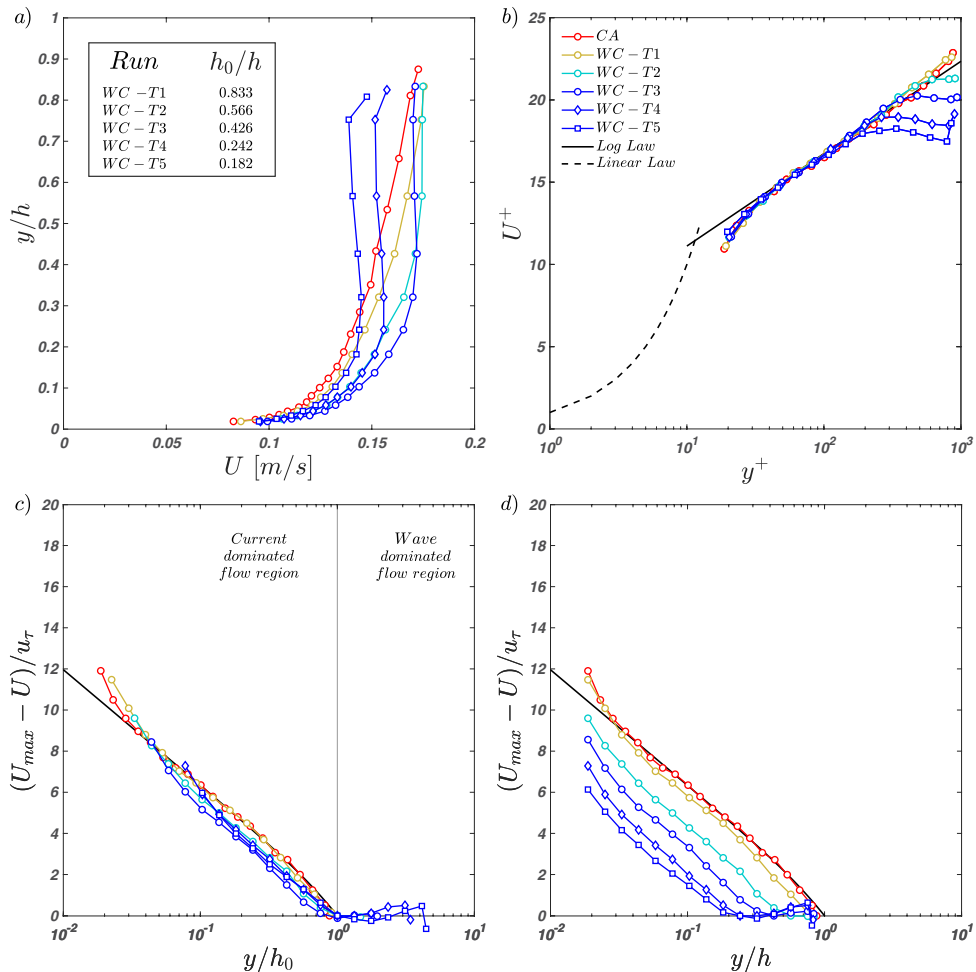


FIGURE 8. Panel (a) shows the vertical profiles of the mean longitudinal velocity for the CA and WC experiments (complete waves plus current signal). In the inset, the normalised values of the proposed outer-length scale h_0 are reported. Panels (b) and (c) show the normalised profiles of the mean longitudinal velocity in inner and outer scaling, respectively. Finally, panel (d) reports the outer-scaled profiles of the mean longitudinal velocity by using the flow depth h as outer scale.

471 respect to the CA case, the vertical profiles pertaining to the WC cases are significantly
 472 different and indicate that waves are responsible for a redistribution of time-averaged
 473 momentum and global shear. In what follows we show that such a redistribution can
 474 be interpreted as the result of waves generating two distinct flow regions in the water
 475 column. The discussion about the existence, scaling and turbulence features of these two
 476 flow regions is at the heart of the whole paper.

477 We begin the analysis by plotting mean velocity profiles following the approach nor-
 478 mally taken in wall turbulence studies, namely in inner and outer scaling (figure 8b–c).
 479 In the following, the superscript ‘+’ refers to the usual inner normalisation $y^+ = yu_\tau/\nu$
 480 and $U^+ = U/u_\tau$, where the u_τ values are listed in table 2. By applying the inner scaling,
 481 the velocity profiles collapse within a narrow interval (figure 8b). Note that the so-
 482 called two-log-profile structure proposed by Grant & Madsen (1979) and experimentally
 483 validated by Fredsøe *et al.* (1999) and Yuan & Madsen (2015) in hydraulically rough-bed

conditions, is not detectable in figure 8(b). This may be attributable to the fact that the Stokes length $l_S = \sqrt{2\nu/\omega}$ - which to some extent quantifies the wave boundary layer thickness δ_w in smooth-bed flows (i.e. $\delta_w = 2 - 4l_S$, Nielsen 1992) - ranges from $5.4 \cdot 10^{-4}$ to $7.6 \cdot 10^{-4}$ m, corresponding to 4.7–6.5 wall units, and therefore it is fully buried within the buffer/viscous layer. Consequently, it is not surprising that the two-log-profile structure was evidenced only for combined wave-current flows over rough-beds, in which case the δ_w is magnified by the bed roughness. Furthermore, it is worth pointing out that the logarithmic region in figure 8(b) is shortened in tests WC–T2 to WC–T5 with respect to the CA case. This result is similar to the finding obtained by Deng *et al.* (2019) when Langmuir cells are present in the water column. This could be connected to the results discussed later in Sections 4.3–4.4.

In the outer scaling there is a reasonably-good collapse of the mean velocity profiles for the CA case and the WC cases if h_0 and $U_{max} - U$ are used as the outer length scale and velocity defect, respectively (see the difference between figure 8c and figure 8d). The quantity h_0 is here defined as the distance from the wall where the mean velocity profile reaches its maximum U_{max} and beyond which it decreases or maintains a constant value (figure 8a). It is also import to highlight that the uppermost measured point in the velocity profiles of tests WC–T4 and WC–T5 were not considered during the determination of h_0 and U_{max} since these points present a sudden increasing in the mean longitudinal velocity probably induced by near surface effects (figure 8a). Given the small number of data points available across the water column, to obtain velocity profiles with higher resolution we interpolated the data using spline functions. Since the maxima locations identified by the cubic spline functions were very close to the maxima in the data points, we estimated the locations of h_0 using the point measurements available (normalised values of h_0 are reported in figure 8a).

Figure 8(b–c) shows that, for each experimental condition, there is a range of elevations where mean velocity profiles nearly collapse both in inner and outer scaling over the log law of the wall (solid lines). Besides CA, data collapse is particularly good for case WC–T1, whereas cases WC–T2, –T3, –T4 and –T5 seem to be shifted slightly downwards. Despite this shift the collapse is satisfactory and suggests the existence of a logarithmic-overlap layer as defined within the remit of asymptotic matching theories (Yaglom 1979). Clearly, the collapse of the data is strongly dependent on the exact location of h_0 (and consequently also the estimation of U_{max}). More refined measurements of velocity profiles are required to further substantiate these results, but the improvement with respect to figure 8(d) is tangible. Another possible explanation for the not perfectly collapse between test WC–T1 and the other WC tests is that a change of the von Kármán coefficient κ occurs. As it is widely recognised (Nagib & Chauhan 2008; Marusic *et al.* 2010), κ varies between different canonical flows (e.g. $\kappa \approx 0.37$ in closed-channel flows, $\kappa \approx 0.384$ in zero-pressure gradient turbulent boundary layers and $\kappa \approx 0.41$ in pipe flows) and it is plausible that, when the effects of the wave motion become relevant, κ change its value from that one observed for the open-channel flow case CA. This observation seems consistent with the results that we show in the following, but, to better elucidate this aspect and remove any doubt a dedicated study should be addressed.

However, the inner-outer scaling of the vertical profiles of the mean longitudinal velocity is noteworthy because: (i) to the best of the authors' knowledge, this is the first time that the existence of a logarithmic layer (which has a profound physical meaning and is very relevant for modelling purposes) in wave-current flows is supported on the basis of arguments that go beyond the simple identification of a log-type shape in the profile of U ; and (ii) the existence of a log profile justifies the use of the Clauser method to estimate the shear velocity in WC experiments. Further support for the existence of a

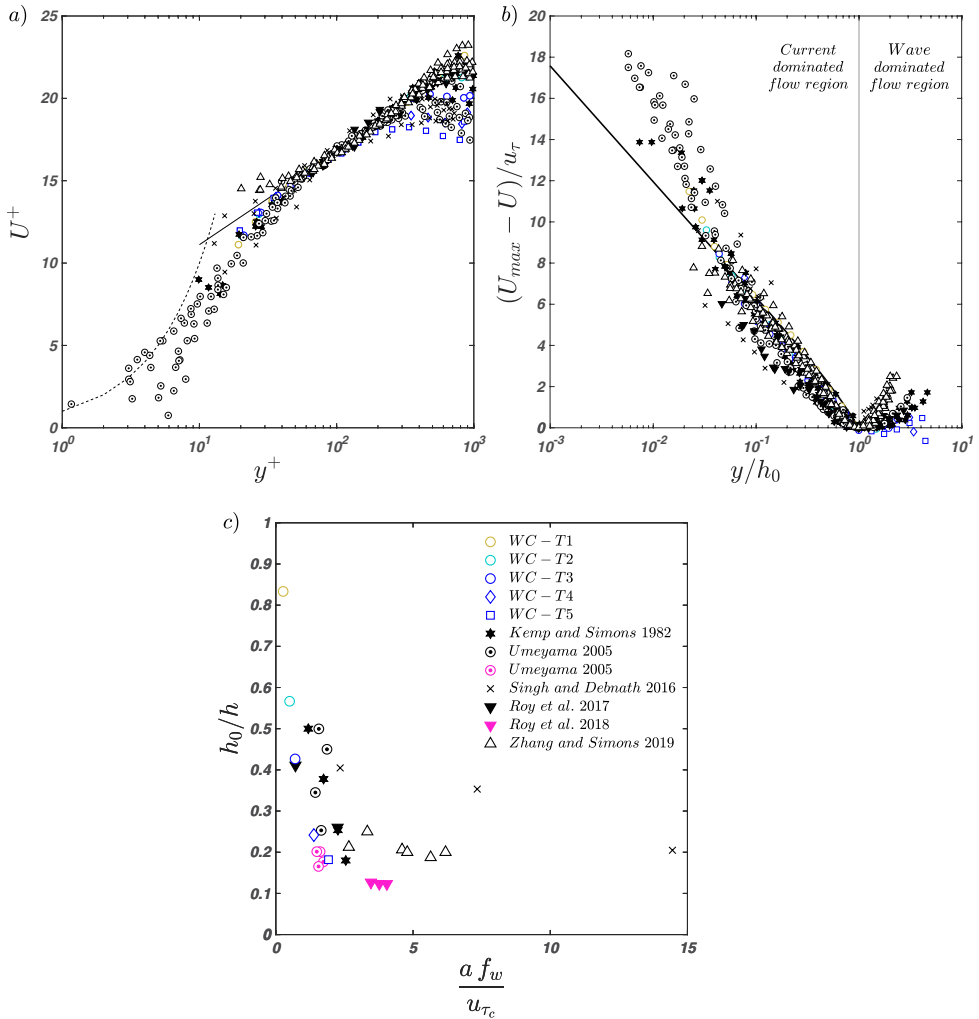


FIGURE 9. Panels (a) and (b) report the normalised profiles of the mean longitudinal velocity of the present results together with data taken from the literature (Kemp & Simons 1982; Umeyama 2005; Singh & Debnath 2016; Roy *et al.* 2017; Zhang & Simons 2019) in inner and outer scaling, respectively. Panel (c) shows the normalised outer-length scale h_0/h as a function of the dimensionless parameter $a f_w/u_{\tau_c}$ for waves following a current (black markers) and waves against a current (magenta markers). The dataset for this latter case were taken from Umeyama (2005) and Roy *et al.* (2018).

534 logarithmic-type layer in the WC experiments will be provided when discussing second
 535 order velocity statistics and spectral analysis.

536 The proposed inner-outer scaling was also employed to available literature data relating
 537 to mean velocity profiles measured in combined wave-current flows with waves following a
 538 current (Kemp & Simons 1982; Umeyama 2005; Singh & Debnath 2016; Roy *et al.* 2017;
 539 Zhang & Simons 2019) to test its universality (figure 9a–b). The value of u_τ and h_0 where
 540 estimated as per the dataset presented herein using the mean velocity profiles extracted
 541 from each referenced paper. As shown in figure 9(a), the velocity profiles collapse very well
 542 in inner scaling but this is somewhat imposed by using the Clauser method to estimate
 543 the friction velocity. In outer scaling, the scatter of data is significant but the velocity

544 profiles seem to cluster around our data (figure 9b). The reasons explaining the observed
 545 scatter can be deduced after interpreting the physical meaning of h_0 as follows.

546 It is herein introduced the concept (further substantiated in the next sections) that the
 547 height h_0 represents a crossover between two different flow regions: (i) the first, between
 548 the bed and h_0 , where the flow is influenced by the presence of waves but retains, to
 549 a good extent, the character of a current (the current-dominated flow region); (ii) the
 550 second, between h_0 and the free-surface, where the flow is mainly controlled by the wave
 551 motion (the wave-dominated flow region; note that also Umeyama (2005) introduced an
 552 inner layer depth included between $y = 0$ and $y = h$ in his work, but without giving
 553 any physical meaning to it). Bearing in mind the nature of the combined wave-current
 554 flows considered in the present study, we propose that h_0 could be dictated by a trade-
 555 off between two competing mechanisms: wave-induced velocities, which, according to
 556 classical wave theories, scale as $a f_w$ and the turbulent velocities imposed by the current,
 557 which notoriously scale with the friction velocity, u_{τ_c} . Therefore, we argue that:

$$\frac{h_0}{h} = F\left(\frac{a f_w}{u_{\tau_c}}; \frac{h}{L}; \frac{\delta_c}{h}\right) \quad (4.1)$$

558 where F is an unknown functional relation and δ_c is the thickness of the current
 559 boundary layer. It is important to recall that in the present work, $a f_w/u_{\tau_c}$ and h/L vary
 560 in the range 0.26 to 1.85 and 0.05 to 0.12 (see table 1 and table 2), respectively, while
 561 $\delta_c/h = 1$ (see Peruzzi *et al.* 2020), hence the validity of Eq. 4.1 is currently limited to
 562 these conditions. From a physical point of view, the shift between the two regions cannot
 563 be as sharp as conceptualised above and it is expected that a sizeable transition zone
 564 exists. However, our reasoning is similar to that of the boundary layer concept, where the
 565 distinction between the turbulent and irrotational flow is set where the mean streamwise
 566 velocity U equals 95–99% of the edge velocity U_e imposed by the potential flow.

567 The proposed scaling (Eq. 4.1) identifies a good trend in the data (listed in table 2
 568 and from the literature) presented in figure 9(c): for increasing values of $a f_w/u_{\tau_c}$,
 569 h_0/h decreases, meaning that the stronger the wave velocities the more the current-
 570 dominated region shrinks towards the bed. Interestingly (and encouragingly), data taken
 571 from the literature (Kemp & Simons 1982; Umeyama 2005; Singh & Debnath 2016; Roy
 572 *et al.* 2017; Zhang & Simons 2019) for the case of waves following a current, while not
 573 collapsing that well with the present data, do show more or less similar trends. The
 574 scatter visible in figure 9(b), might be the result of two factors. First, as already pointed
 575 out, combined wave-current flows are possibly non-equilibrium flows whose scaling is
 576 implicitly not universal. Second, the literature data refer to flow conditions whereby waves
 577 are superimposed to currents whose ratio between depth and boundary layer thickness
 578 (equal to 1 for the experimental data pertaining to the present paper) is not the same
 579 among different experiments, which are therefore not fully comparable. Furthermore,
 580 in figure 9(c) are reported the data from Umeyama (2005) and Roy *et al.* (2018) for
 581 waves against a current situation. The values of h_0/h were determined by using the more
 582 general procedure for the identification of h_0 explained in the last part of Section 4.2.
 583 These new data seem consistent with our theoretical framework, although they display
 584 a downward shift from the waves following a current data. It may be possible that in
 585 the waves against current environment, the current-dominated region is further shrunken
 586 but, at the moment, we do not have sufficient information to comment on this.

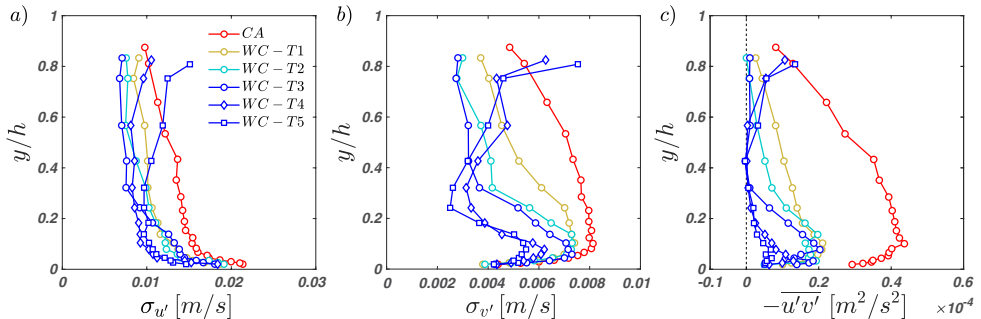


FIGURE 10. Profiles of the dimensional Reynolds stresses. In panel (a) $\sigma_{u'}$ is the standard deviation of the turbulent longitudinal velocity component; in panel (b) $\sigma_{v'}$ is the standard deviation of the turbulent vertical velocity component; in panel (c) $-\overline{u'v'}$ is the covariance between the turbulent components of the longitudinal and vertical velocities.

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4.2. Reynolds stresses

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Because we are mainly interested in the effect of waves on turbulence, in the following, we focus our attention on the Reynolds stresses computed from the turbulent signal only, we encourage the readers interested in the Reynolds stresses of the wave signal to read [Peruzzi \(2020\)](#).

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It is convenient to begin commenting the Reynolds stresses (as obtained from the turbulence velocity signal extracted using the EMD) plotted in dimensional form as this allows for comparisons with data previously presented in the literature. Figure 10(a–c) indicates that the Reynolds stresses for the WC cases deviate considerably from the benchmark CA case. In agreement with other experimental studies (e.g. [Umeyama 2005](#); [Singh & Debnath 2016](#)), the normal ($\sigma_{u'}$ and $\sigma_{v'}$) and shear Reynolds stresses ($-\overline{u'v'}$) are damped by the presence of the wave motion (in particular the shear component, which shows a dramatic reduction in magnitude). In accordance with what observed from previous studies, in the near bed region the profiles of normal and shear Reynolds stresses retain a peak (not visible for $\sigma_{u'}$ due to spatial resolution issues) as observed for the CA flow. Away from the bed, the shape of the profiles is severely altered by the passage of waves. As observed by [Umeyama \(2005, 2009a,b\)](#) and [Roy et al. \(2017\)](#), such profiles tend to become flatter or, for the experiments WC–T4 and WC–T5, associated with a switch in sign of their vertical gradient. Finally, the shear Reynolds stress $-\overline{u'v'}$ is always positive throughout the water column (indicating a downward turbulent momentum transport) and for cases WC–T3 to WC–T5 becomes null at $y/h \approx 0.4$.

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Clearly, it is extremely difficult to infer properties of turbulence by assessing dimensional quantities as reported in figure 10(a–c). As shown in the following text, the use of an appropriate scaling is more revealing.

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The second-order moments in inner and outer scaling are reported in figure 11(a–f). On the one hand, the Reynolds stress profiles do not either collapse or stratify well when plotted in inner scaling (figure 11a–c). On the other hand, the outer scaling unveils interesting features when h_0 is used as the outer length-scale (figure 11d–f): (i) for the WC experiments, $\sigma_{u'}/u_\tau$ are generally slightly lower with respect to the CA case but collapse fairly well in the current dominate region and show no obvious dependence to wave properties (figure 11d); (ii) the $\sigma_{v'}/u_\tau$ profiles are damped significantly with respect to the CA case but, contrary to $\sigma_{u'}/u_\tau$, show a clear dependence on the parameter $a f_w/u_{\tau c}$ (figure 11e); and (iii) the $-\overline{u'v'}/u_\tau^2$ profiles decrease with y/h_0 , tending to zero for $y/h_0 \approx 1$ (figure 11f).

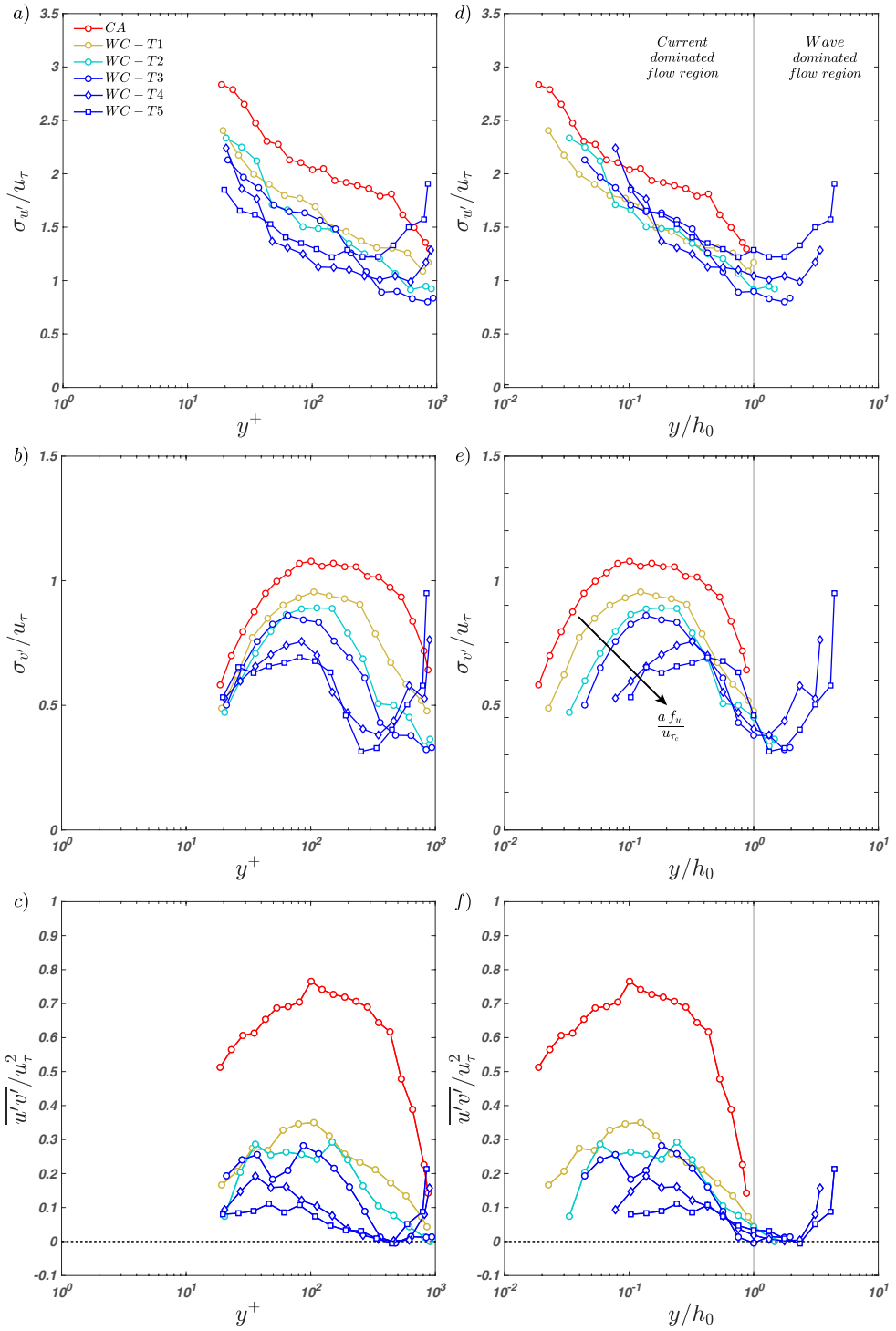


FIGURE 11. Profiles of non-dimensional Reynolds stresses. Panels (a)–(c), inner scaling; panels (d)–(f) outer scaling.

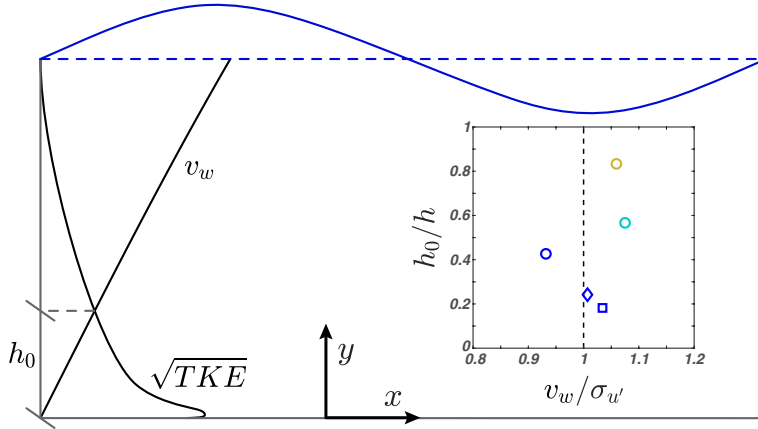


FIGURE 12. Representation of the vertical profiles of the maximum amplitude of the wave-induced vertical velocity v_w , according to the linear wave theory, and of the square root of TKE, where $\text{TKE} = 0.5(\sigma_{u'}^2 + \sigma_{v'}^2 + \sigma_{w'}^2)$. Phenomenologically, we expect that h_0 is located at the elevation where these two quantities are comparable. The inset reports the normalised h_0 (obtained as the maximum of the mean velocity profiles) as a function and of $v_w/\sigma_{u'}$ for the five runs. Since the spanwise velocity w was not measured, $\sqrt{\text{TKE}}$ was estimated as $\sigma_{u'}$, as commonly done in turbulence wall flows (Pope 2000).

621 Among the investigated Reynolds stress profiles, the one that seems to respond more
 622 consistently to different wave forcing is $\sigma_{v'}/u_\tau$, which decreases with increasing $a f_w/u_{\tau_c}$
 623 (figure 11e). Moreover, $\sigma_{v'}/u_\tau$ profiles display a plateau (as encountered in canonical
 624 wall flows) whose extent reduces with increasing $a f_w/u_{\tau_c}$, probably because the whole
 625 current-dominated flow region also shrinks in size (i.e. h_0/h reduces, see figure 9c). Inter-
 626 estingly, in canonical wall flows this plateau is normally associated with the occurrence of
 627 attached eddies (Nickels *et al.* 2007) and, as surmised from the analysis of mean velocity
 628 profiles (figure 8b–c), of a logarithmic layer. The existence of attached eddies seems
 629 therefore to be another feature of canonical wall-flows which resists to the perturbing
 630 action of waves within the current-dominated flow region. This hypothesis will be further
 631 corroborated by spectral analysis in Section 4.3.

632 The vertical profiles of the Reynolds stresses scaled with h_0 all display a clear change
 633 in behaviour at $y/h_0 = 1$, hence further substantiating that h_0 is a crossover length scale
 634 between two flow regions dominated by a significantly different physics. While there is
 635 now reasonably good evidence supporting the hypothesis of h_0 being a relevant length
 636 scale in combined wave-current flows, its definition is admittedly unsatisfactory. As a
 637 matter of fact, the elevation where mean velocity profiles display a maximum cannot be
 638 considered a general definition for h_0 because, for example, it would not be valid for the
 639 analysis of waves opposing currents, where such a maximum does not appear (Kemp &
 640 Simons 1983; Klopman 1994; Umeyama 2005; Roy *et al.* 2018). In an attempt to overcome
 641 this shortcoming we provide a more general criterion as follows.

642 So far it has been argued, although fairly vaguely, that h_0 is dictated by a competing
 643 mechanism between wave motion and current-induced turbulence (see figure 9c). Let
 644 us now consider wave and current flows individually. According to irrotational wave
 645 theory, wave-induced motion progressively reduces with decreasing y mainly because of
 646 the vertical velocity component dying off in response to the impermeability condition
 647 imposed by the bed (figure 12). Conversely, the turbulent kinetic energy (TKE) of the
 648 current, which can be taken as a good indicator of turbulent motion intensity, increases

with reducing distance from the bed. Hence, it is reasonable to assume that the aforementioned competing mechanism results into h_0 corresponding to the elevation where the square root of current-induced TKE and the wave-induced vertical velocity become comparable (figure 12). Consistently with this hypothesis, we report that the values of h_0 as identified from mean velocity profiles correspond to a very good approximation to the elevation where the maximum amplitude of the wave-induced vertical velocity component $v_w = a\omega \sinh ky / \sinh kh$ (as estimated from linear wave theory and recalling the coordinate system shown in figure 1d) equals σ'_u of the current alone CA case that, in wall flows, is known to be a very good estimator of $\sqrt{\text{TKE}}$ (see e.g. Pope 2000). Note that h_0 relates equally well to the elevation where $v_w/\sigma_{v'}$ is about 2 because of the scaling of velocity variances in the CA flow (i.e. in turbulent wall flows $\sigma_{u'}/\sigma_{v'}$ is nearly equal to 2 over the entire outer region).

The trustworthiness of the criterion $v_w/\sigma_{u'} = 1$ for the identification of h_0 is supported by the results reported in figure 9(c), where the values for the waves against current were determined in this way. We believe that this criterion for the identification of h_0 is of more general validity and more physically based than the one based on the maxima in mean velocity profiles; however, we do realise that more data pertaining to a wider range of flow conditions is required to verify its reliability.

4.3. Spectral analysis: on large-scale structures in the current-dominated flow region

It is now interesting to investigate how, with respect to the benchmark CA case, waves affect velocity spectra and hence how turbulent kinetic energy components distribute over different length scales in the WC experiments. By using the Taylor frozen-turbulence hypothesis (Taylor 1938), the 1-D power spectrum of the longitudinal velocity component $E_{xx}(k_x)$ in the wavenumber domain k_x can be estimated from its frequency counterpart $E_u(f)$ by using $k_x = 2\pi f/U(y)$ and $E_{xx}(k_x) = E_u(f)U(y)/2\pi$, where $U(y)$ is the local mean velocity. The 1-D power spectrum of the vertical velocity component $E_{yy}(k_x)$ can be similarly estimated with the appropriate modifications. Since the spectral distortion induced by the Taylor frozen-turbulence hypothesis is stronger in the near-wall region and weaker above $y/h = 0.1$ (Nikora & Goring 2000), in the following the results are mainly discussed for $y/h \geq 0.1$.

Figure 13(a–f) and figure 14(a–f) report 1-D pre-multiplied spectra of the complete signal (i.e. the original wave plus current signal) of the longitudinal and vertical velocity component, respectively. Note that the panels b–f in figure 13 and in figure 14 refer to the WC experiments where spectral peaks associated with characteristic wavenumbers of the imposed waves are much more energetic than the remaining part of spectral estimates. For convenience, in these figures such peaks are visually cut off (and neighbour spectral estimates plotted in light colour) to allow for a more comfortable analysis of the spectral estimates at turbulence-related energy levels. It is also important to highlight that the Taylor frozen-turbulence hypothesis used to plot figure 13(a–f) and figure 14(a–f) is valid for spectral estimates associated with turbulent eddies. Frequencies associated with waves' motion should be transformed into wavenumbers using the waves' celerity $C = L/T$. That is why there is a mismatch between wave-induced peaks in figure 13(a–f) and figure 14(a–f) and actual wavenumbers of the waves as reported in table 2.

The pre-multiplied spectra in the CA experiment display the characteristic double-peak shape (green and red arrows in figure 13a) that was detected both in smooth (Duan *et al.* 2020; Peruzzi *et al.* 2020) and rough-wall (Cameron *et al.* 2017) open-channel flows. The peak at the higher wavenumber is usually associated with the passage of so-called Large-Scale Motions (LSMs) whereas the peak at the lowest wavenumbers is associated with the occurrence of VLSMs. For experiment WC–T1, VLSM peaks can still be detected in the

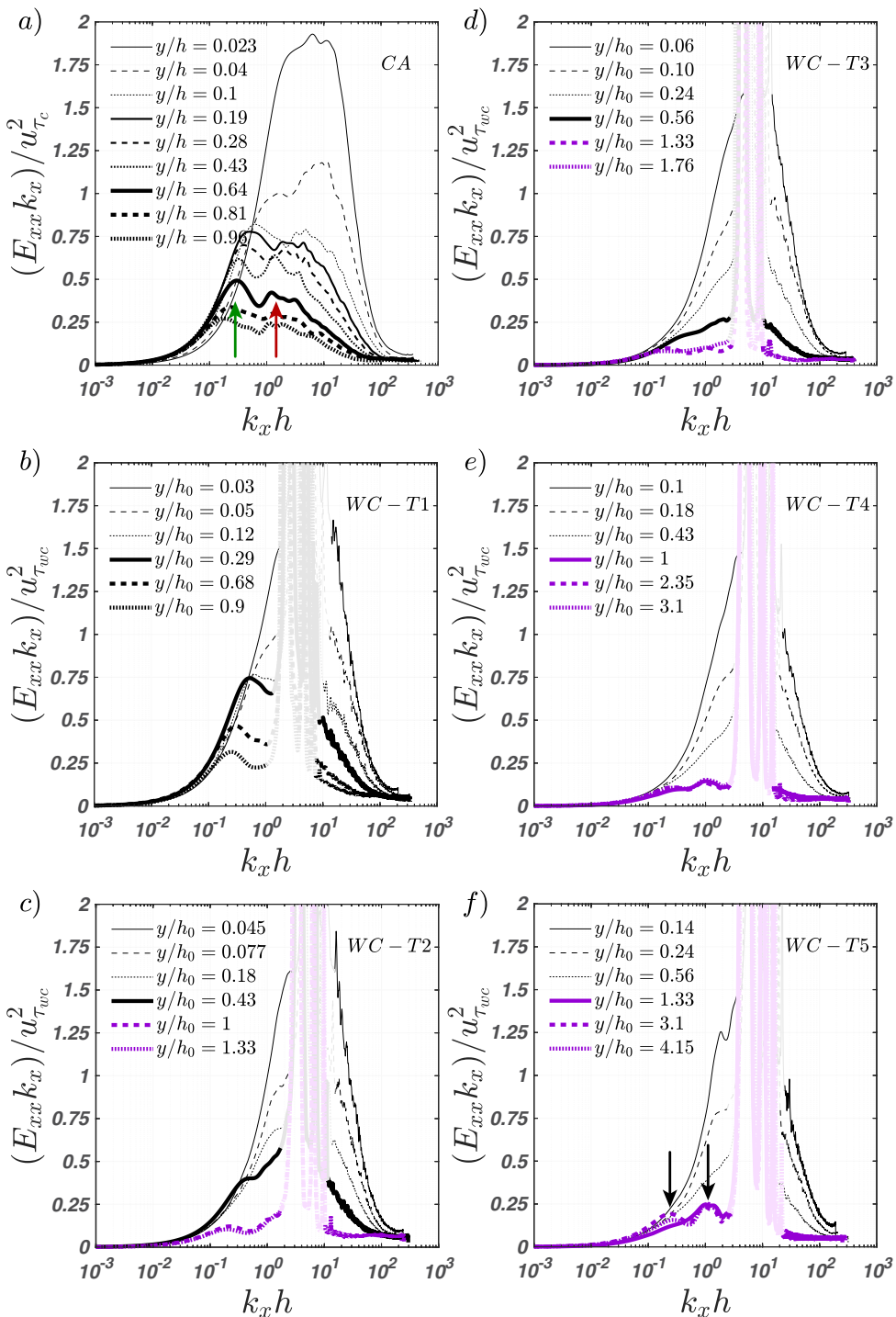


FIGURE 13. Outer-scaled pre-multiplied 1-D spectra of the longitudinal velocity component (complete wave plus current signal). Each panel reports spectra at different elevations for one experimental condition. Black lines identify vertical elevations below h_0 (i.e. in the current-dominated flow region), whereas purple lines above it (i.e. in the wave-dominated flow region). Red and green arrows in panel (a) identify spectral peaks associated with LSMs and VLSMs, respectively. Black arrows in panel (f) identify spectral peaks presumably associated with Langmuir-type turbulence in WC experiments; peaks at similar wavenumbers are also observed in panels (c), (d) and (e). The 95% confidence interval for the pre-multiplied one-dimensional spectra is approximately 0.91 to 1.1 times $(E_{xx}k_x)/u_{\tau_c}^2$.

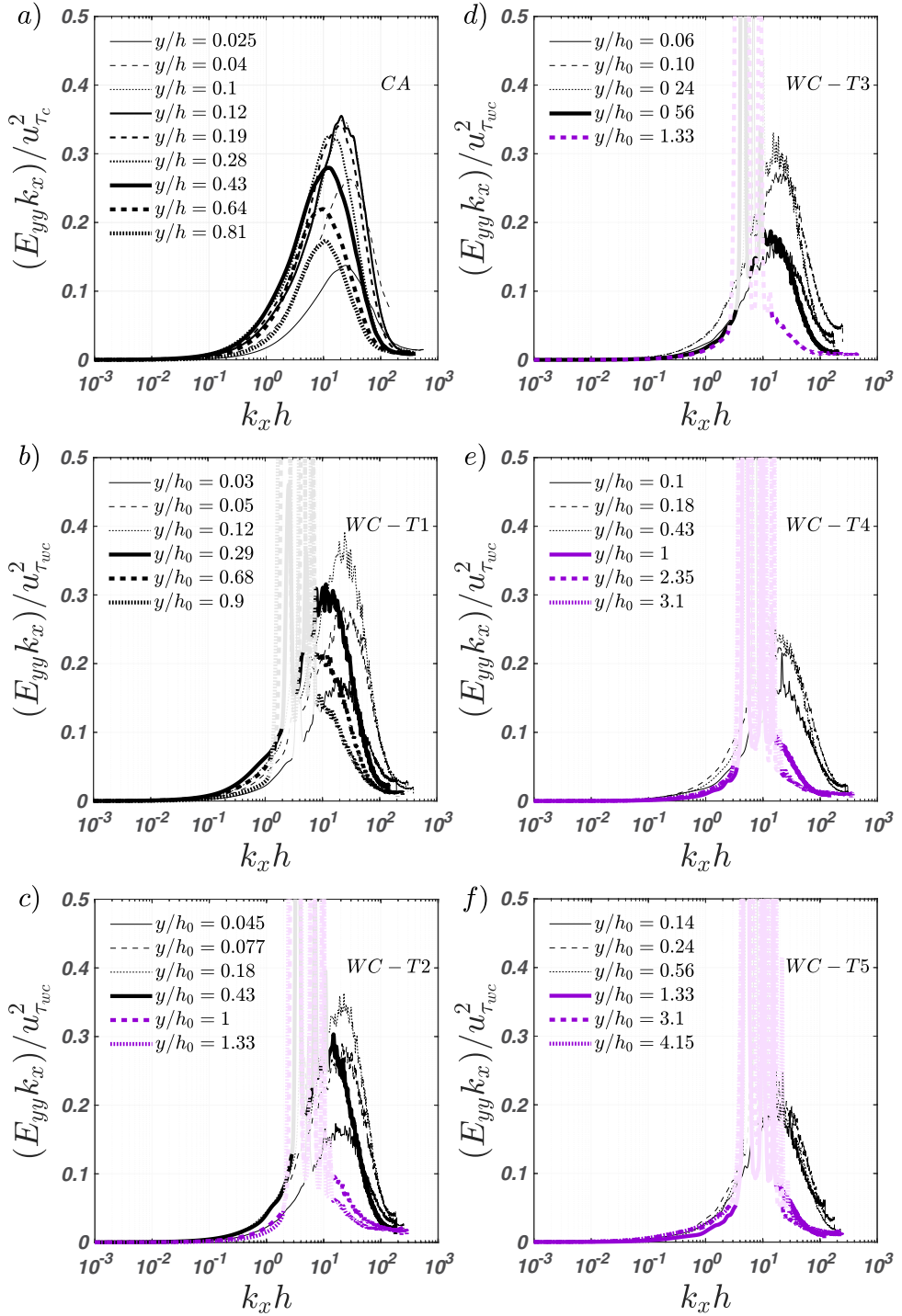


FIGURE 14. Outer-scaled pre-multiplied 1-D spectra of the vertical velocity component (complete wave plus current signal). Each panel reports spectra at different elevations for one experimental condition. Black lines identify vertical elevations below h_0 (i.e. in the current-dominated flow region), whereas purple lines above it (i.e. in the wave-dominated flow region).

698 pre-multiplied spectra, probably because wave motion is significantly less intense than
 699 turbulence, i.e. $a f_w/u_{\tau_c}$ is very small (figure 13b, table 2). For the remaining WC cases,
 700 instead, wave motion is strong enough (i.e. $a f_w/u_{\tau_c}$ is large enough) to suppress VLSMs
 701 (figure 13c–f, table 2). It is possible to argue that the critical value for VLSMs suppression
 702 should be in between that of WC–T1 and WC–T2, i.e. 0.25 and 0.5 (see table 2). For
 703 what concerns LSMs, they cannot be distinguished in any of the WC experiments because
 704 spectral peaks due to waves occupy the wavenumbers where LSMs would be expected to
 705 display their peaks (compare e.g. figure 13a and figure 13b). It is therefore difficult to
 706 assess whether LSMs are suppressed or not by the passage of waves.

707 The reason why VLSMs (and possibly LSMs) are suppressed is difficult to identify
 708 with the data presented. However, it should be noted that, at the investigated CA flow-
 709 conditions, LSMs and VLSMs are associated with wavelengths of $\approx 5h - 7h$ and \approx
 710 $20h - 25h$ respectively. These values are comparable with the spatial length scale imposed
 711 by the wave motion (the wave length L), which is $\approx 8h - 20h$ (depending on the run,
 712 see table 1), hence it is plausible that, provided $a f_w/u_{\tau_c}$ is large enough, waves strongly
 713 interact and possibly suppress turbulence structures of similar length.

714 In the pre-multiplied spectra of the vertical velocity component, as measured in the
 715 current-dominated flow region (i.e. $y/h_0 < 1$), there is a clear scale separation between
 716 peaks due to energetic turbulent structures and peaks imposed by waves (figure 14b–f),
 717 which allows for some interesting observations. For all WC experiments, the peaks
 718 caused by turbulence structures occur over the same range of wavenumbers as in the
 719 CA experiment, where, as per other canonical wall flows, they are usually considered as
 720 a characteristic trait of attached eddies (Baidya *et al.* 2017). This result further confirms
 721 what surmised from the analysis of the $\sigma_{v'}/u_{\tau}$ profiles: attached eddies resist to waves'
 722 perturbations and continue to populate the current-dominated flow region. Conversely,
 723 in the wave-dominated flow region (i.e. $y/h_0 > 1$) there is no scale separation between
 724 turbulence and waves (i.e. it is impossible to distinguish between peaks associated with
 725 turbulence and waves), which suggests that turbulent velocity fluctuations are associated
 726 with mechanisms possibly powered by waves.

727 4.4. Spectral analysis: on large-scale structures in the wave-dominated flow region

728 The pre-multiplied spectra pertaining to the wave-dominated flow region (purple lines)
 729 also show some unexpected features (figure 13c–f). They display either one or two peaks
 730 (or bumps) at rather low wavenumbers (see black arrows in panel f), suggesting that the
 731 wave-dominated flow region hosts turbulence structures at scales comparable to LSMs
 732 and VLSMs (the wavelength λ_x of these structures is equal to about $25h$ and $6h$ for
 733 the peak at the lowest and highest wavenumber, respectively). This is rather counter-
 734 intuitive because in the current-dominated flow region such structures are suppressed
 735 by waves and it is surprising to see them in the wave-dominated region. With the data
 736 set presented, it is rather difficult to discuss the physical mechanisms underpinning the
 737 formation of such structures; however, for the sake of discussion and to identify future
 738 research directions, some hypotheses can be made.

739 Towards this end, it is worth recalling the study by Huang & Mei (2006), which
 740 reports a linear stability analysis of turbulent open-channel flows over smooth beds
 741 superimposed to waves, exactly as in the present study. Besides linearising the equation
 742 of motion and boundary conditions at the free surface and at the bed surface, Huang
 743 & Mei (2006) made the following assumptions: (i) the dimensionless water depth was
 744 set of order unity $kh = \mathcal{O}(1)$, (ii) the wave steepness $\epsilon = ka$ was small, and (iii) the
 745 wave orbital velocity was set comparable to the current velocity; all these conditions
 746 are reasonably met in our experiments (table 2). Interestingly, and in line with our

747 experimental results, their stability analysis identified two large-scale unstable modes.
 748 These modes were associated with cellular structures with longitudinal vorticity, akin
 749 to Langmuir-type turbulent cells. Huang & Mei (2006) point out that, analogously to
 750 Langmuir turbulence, the key requirements for the production of longitudinal vorticity,
 751 and hence of the two observed unstable modes, are a source of vertical vorticity (e.g. any
 752 spanwise perturbation of the longitudinal velocity) and a horizontal shear stress induced
 753 by vertical gradients of longitudinal velocities. The vertical vorticity interacts with the
 754 Stokes drift shear to generate longitudinal vorticity through vortex tilting and stretching.
 755 The resulting spanwise gradient of the vertical velocity component interacts with the
 756 mean shear imposed by the current to generate further vertical vorticity (presumably via
 757 vortex stretching) to sustain the whole process of longitudinal vorticity generation.

758 The self-sustained process proposed by Huang & Mei (2006) might explain the two
 759 peaks observed in figure 13(c–f). However, Huang & Mei (2006) did not estimate
 760 the characteristic longitudinal wavenumber of the detected instabilities. This makes it
 761 difficult to carry out a full and direct comparison between their theoretical results and
 762 the present experimental data (i.e. the wavenumber at which spectral peaks occur in
 763 figure 13c–f). The recent work by Xuan *et al.* (2019), though, indicates that Langmuir
 764 turbulence (which is not the one discussed herein and by Huang & Mei (2006), but it does
 765 share some similarities) occurs in the form of elongated eddies of length equal to eight
 766 times their width. Assuming that the cells in the present experiments are circular and
 767 filling the entire wave-dominated region, this implies that their width is about $h - h_0$ and
 768 hence about $0.2 - 0.5h$ (see figure 12). This means that the estimates provided by Xuan
 769 *et al.* (2019) are close to those of the peak observed at $k_x h = \mathcal{O}(1)$ in figure 13(c–f).
 770 Furthermore, Deng *et al.* (2020), through a wall-modelled Large Eddy Simulation (LES)
 771 of shallow-water Langmuir turbulence with a very large computational domain ($\approx 100h$),
 772 reveal streamwise streaks induced by Langmuir cells that meander in the streamwise
 773 direction with a wavelength of around $25h$, in accordance with the peak observed at
 774 $k_x h = \mathcal{O}(0.1)$ in figure 13(c–f). As it is well known, one-dimensional velocity spectra
 775 measure the spanwise meandering frequency rather than the actual wavelength of large-
 776 scale turbulent structures (Hutchins & Marusic 2007), and the meandering configuration
 777 reported in Deng *et al.* (2020) well support the spectral footprints herein reported. Also
 778 in line with our experimental data is the fact that Huang & Mei (2006) observed that
 779 the unstable modes occur only for wave steepness ϵ greater than 0.02 and the larger ϵ the
 780 stronger their growth rate. In our experiments, $\epsilon < 0.02$ only for the case WC–T1 and
 781 ϵ increases from WC–T2 to WC–T5 (table 2). Remarkably, all the WC cases, excepts
 782 for WC–T1, present evidence of instabilities in line with the modes reported by Huang
 783 & Mei (2006) in the wave-dominated flow region (figure 13c–f). It is also worth noting
 784 that WC–T5 is characterised by the highest value of ϵ and the most pronounced spectral
 785 peaks at low wavenumbers (see figure 13f).

786 To the authors' opinion, the experimental data presented herein combined with the
 787 theoretical analysis proposed by Huang & Mei (2006) provide clues to support the idea
 788 that, in the wave-dominated flow region, turbulence is organized in eddies similar to
 789 Langmuir cells. These findings are also in line with the experimental results of Nepf &
 790 Monismith (1991), who reported the presence of longitudinal vortices arising through
 791 wave-current interaction.

792 5. Discussion

793 Note that in Section 4.3, we have argued that the suppression of VLMSs in the current-
 794 dominated region is controlled by $a f_w / u_{\tau_c}$ whose critical value lies between 0.26 and

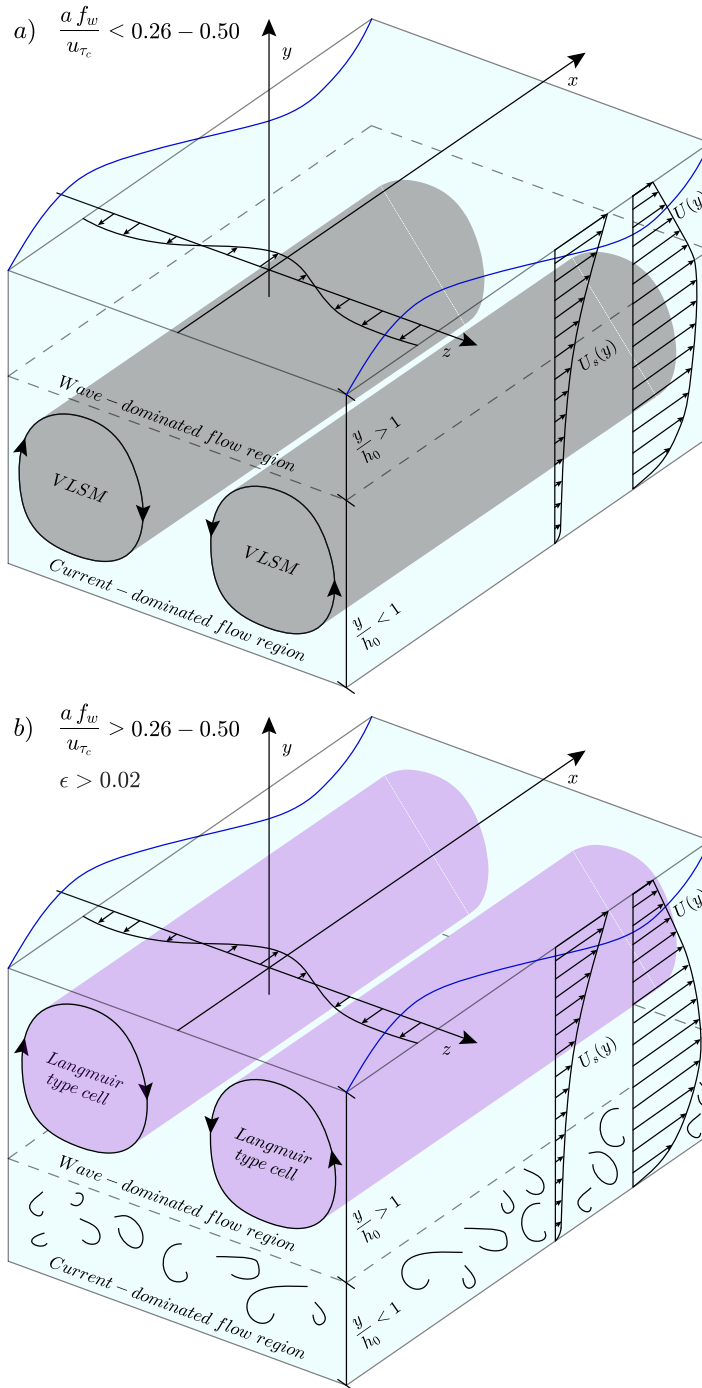


FIGURE 15. Representation of large-scale turbulence phenomenology in combined wave-current flows as observed in the present paper: (a) for cases where $a f_w/u_{\tau_c} < 0.26 - 0.5$; (b) for cases where $a f_w/u_{\tau_c} > 0.26 - 0.5$ and $\epsilon > 0.02$. The vertical profiles of the longitudinal mean velocity U and the Stokes drift U_s are not to scale.

795 0.50. Instead, according to Huang & Mei (2006), the presence of large-scale structures
 796 in the wave-dominated region is controlled by wave steepness (i.e. ϵ should exceed 0.02).
 797 Nevertheless, for a wave-dominated region to exist also large values of $a f_w/u_{\tau_c}$ are
 798 required, so we expect that both non-dimensional parameters should be employed for
 799 the diagnostics of Langmuir-type turbulence in combined wave-current flows. Figure 15
 800 is a sketch where these concepts are graphically summarised. For low values of $a f_w/u_{\tau_c}$
 801 (panel a), the wave-dominated region is thin and VLSMs persist in the current-dominated
 802 region. When $a f_w/u_{\tau_c}$ attains higher values (panel b), VLSMs in the current-dominated
 803 region vanish and, provided $\epsilon > 0.02$, Langmuir-type turbulence appears in the wave-
 804 dominated region.

805 It is now worthy to recall some features pertaining to neutrally-stratified shallow-water
 806 Langmuir turbulence, where Langmuir cells engulf the entire water column, impacting the
 807 vertical mixing and the bottom boundary layer (Tejada-Martínez & Grosch 2007; Tejada-
 808 Martínez *et al.* 2012; Sinha *et al.* 2015; Xuan *et al.* 2019; Deng *et al.* 2019, 2020). In this
 809 circumstance, the current is wind-driven generated by surface stresses τ_s (figure 16a). A
 810 key dimensionless parameter to understand if the interaction between the wind-driven
 811 shear current and the Stokes drift current induced by surface gravity waves is able to
 812 generate Langmuir circulation (and the associated turbulence), is the turbulent Langmuir
 813 number defined by McWilliams *et al.* (1997) as:

$$La_t = \sqrt{\frac{u_{\tau_{wind}}}{U_s(h)}} \quad (5.1)$$

814 i.e., it is the ratio between the friction velocity induced by the wind ($u_{\tau_{wind}} = \sqrt{\tau_s/\rho}$,
 815 where ρ is the water density) and the surface Stokes drift velocity $U_s(h)$. Following
 816 Tejada-Martínez & Grosch (2007), the characteristic surface Stokes drift velocity is
 817 defined as:

$$U_s(h) = \omega ka^2 = C\epsilon^2 \quad (5.2)$$

818 where $C = \omega/k = L/T$ is the wave celerity. Considering the results coming from LES
 819 simulations (Li *et al.* 2005; Sinha *et al.* 2015; Deng *et al.* 2019), the transition between
 820 shear turbulence to Langmuir turbulence occurs for $La_t < 1$, i.e. when the wave motion
 821 start to overcome the wind-induced current.

822 In our situation, Eq. 5.1 could not be straightforwardly used to verify the occurrence of
 823 the Langmuir-type cells in the wave-dominate region since the shear turbulence is driven
 824 by a different mechanism (figure 16b). In particular, since the turbulence is generated
 825 at the bed, the shear velocity of the current is defined as $u_{\tau_c} = \sqrt{\tau_0/\rho}$, where τ_0 is
 826 the bed shear stress. Thus, it is necessary to modify the La_t number with respect to
 827 the type of current (wave-drive or pressure-driven) present in the flow. In the context
 828 of pressure-driven currents, we introduce a slightly different turbulent Langmuir number
 829 La_t as:

$$La_t = \sqrt{\frac{u_{\tau_c}}{U_s(h)}} \quad (5.3)$$

830 To understand when the Langmuir-type cells represented in figure 15(a) start to occur,
 831 we manipulate the two conditions previously identified, i.e. $a f_w/u_{\tau_c} > 0.26 - 0.5$ and
 832 $\epsilon > 0.02$. Considering the term $a f_w/u_{\tau_c}$, it can be manipulated to obtain:

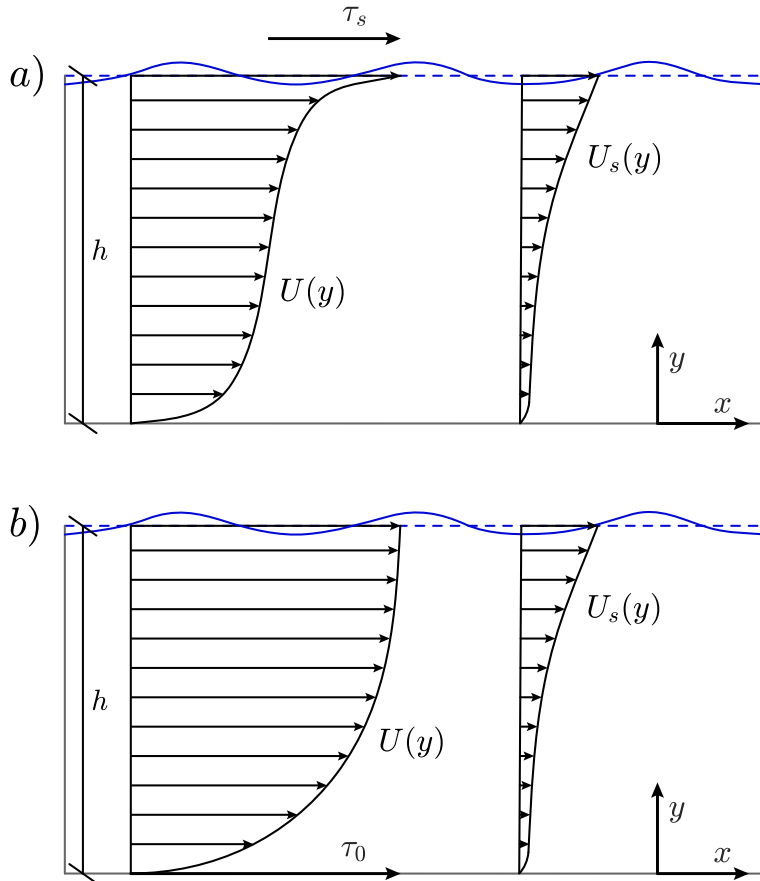


FIGURE 16. Surface gravity waves interacting with: (a) wind-driven current; (b) pressure-driven current. The vertical profiles of the longitudinal mean velocity U and the Stokes drift U_s are not to scale.

$$\frac{a f_w}{u_{\tau_c}} = \frac{a f_w C}{u_{\tau_c} C} = \frac{a C}{u_{\tau_c} L} = \frac{C \epsilon}{2\pi u_{\tau_c}} = \frac{C \epsilon^2}{2\pi \epsilon u_{\tau_c}} = \frac{1}{2\pi \epsilon La_t^2} \quad (5.4)$$

833 Introducing $A = \begin{pmatrix} 0.26 \\ 0.50 \end{pmatrix}$, the first condition can be rewritten as:

$$\frac{1}{\epsilon La_t^2} > 2\pi A \quad \longrightarrow \quad La_t < \sqrt{\frac{1}{2\pi \epsilon A}} \quad (5.5)$$

834 Assuming for simplicity $\epsilon = 0.02$ (from the second condition), we finally obtain:

$$La_t < 4 - 5.53 \quad (5.6)$$

835 Interestingly, using the values reported in table 1 and table 2 to compute La_t , we obtain
 836 $La_t = 8.18, 4.25, 2.88, 1.44, 0.96$ for cases from WC-T1 to WC-T5, respectively. It is
 837 not surprisingly that the threshold range obtained in Eq. 5.6 is higher than 1 since
 838 u_{τ_c} is evaluated at the wall and does not represent the actual balance of forces at the
 839 free-surface between the wave motion and the current.

840 It is important to highlight that, in open-channel flume facilities the main source of
 841 vertical vorticity is the boundary layers that develop at the sidewalls of the channel (Nepf

842 & Monismith 1991). Thus the formation of the Langmuir-type cells could be facilitated
 843 with respect to a natural situation, where pressure-driven currents (e.g. tide currents)
 844 interact with surface gravity waves in a less confined environment.

845 6. Conclusions

846 A new set of experiments featuring LDA velocity measurements was carried out in a
 847 smooth-bed turbulent open-channel flow where surface waves with various frequency f_w
 848 and amplitude a were superimposed on a current.

849 Due to the irregularity of the wave motion generated within the experimental flume
 850 facility, the separation of the turbulent and wave components from the original signal
 851 was achieved by employing the Empirical Mode Decomposition (EMD), a data analysis
 852 technique that is being increasingly used in coupled wave-currents flows and represents
 853 a suitable technique for both laboratory and field applications (Schmitt *et al.* 2009; Qiao
 854 *et al.* 2016), hence allowing for future data-set comparisons.

855 The experimental results presented in Section 4 provide an interesting picture about
 856 turbulence in open-channel flows perturbed by following waves.

857 As surmised in the literature, but never really demonstrated, a genuine logarithmic-
 858 overlap layer seems to occur in WC flows. This is corroborated by finding that, for a
 859 range of elevations, mean velocity data collapse both in inner and outer scaling, provided
 860 that the maximum velocity U_{max} of the profile and the elevation at which it occurs
 861 (h_0) are employed to define the velocity defect and the outer length-scale, respectively.
 862 Interestingly, h_0 also corresponds to the elevation where the Reynolds shear stress reduces
 863 to zero, which endorses the hypothesis of h_0 being a length-scale akin to a boundary layer
 864 depth below which the flow scales similarly to canonical wall-flows. It follows that h_0 can
 865 be used to identify two flow regions: a current-dominated flow region in the lower part of
 866 the water column ($y/h_0 < 1$; figure 15), and a wave-dominated flow region in the upper
 867 part ($y/h_0 > 1$; figure 15).

868 The profiles of all Reynolds stresses in the current-dominated flow region display some
 869 similarities with the profiles occurring in canonical wall flows even though they are not
 870 free from wave effects. Similarities include the occurrence of a plateau in $\sigma_{v'}/u_\tau$, which
 871 testifies the presence of attached eddies and, although indirectly, confirms the presence of
 872 a genuine logarithmic layer as surmised from mean velocity profiles. Wave effects include
 873 a reduction in Reynolds stress magnitude with respect to the CA case. The reduction
 874 is more evident for $\sigma_{v'}/u_\tau$ and $-\overline{u'v'}/u_\tau^2$ than for $\sigma_{u'}/u_\tau$. Interestingly, the damping of
 875 $\sigma_{v'}/u_\tau$ is found to be strongly correlated to the relative magnitude of wave velocities
 876 with respect to turbulence, i.e. a parameter identified as $a f_w/u_{\tau_c}$, which therefore seems
 877 to be a key non-dimensional parameter to characterise combined wave-current flows.
 878 This is corroborated by the fact that the relative depth h_0/h correlates fairly well with
 879 $a f_w/u_{\tau_c}$. Indeed, h_0/h is found to reduce as $a f_w/u_{\tau_c}$ increases, meaning that the current-
 880 dominated flow region shrinks towards the bed, leaving space to an overlying region where
 881 turbulence is controlled by wave motion, i.e. the wave-dominated flow region (figure 15).
 882 A more in depth analysis of the data reveals that h_0 corresponds to the elevation where
 883 the vertical component of the waves' motion (as obtained from irrotational wave theories)
 884 equals $\sigma_{u'}$ of the CA case, which is a good proxy for the square root of the turbulent
 885 kinetic energy (Pope 2000). The implication of this result is twofold: first, it suggests that
 886 it is through vertical motion that waves compete with turbulence to dictate h_0 ; second, it
 887 provides a more general way to identify h_0 which can be applicable also for flow conditions
 888 displaying no maximum in the mean velocity profile (which was the criterion used herein)
 889 as in the case of waves opposed to currents.

890 Spectral analysis provided important information about the structure of turbulence
 891 in both current- and wave-dominated regions. Pre-multiplied spectra of the vertical
 892 velocity component provided support for the existence of attached eddies in the current-
 893 dominated region as inferred from the vertical profiles of $\sigma_{v'}/u_{\tau}$. The pre-multiplied
 894 spectra related to the longitudinal velocity component reveal that, in the current-
 895 dominated region, waves tend to suppress VLSMs, whereas in the wave-dominated region
 896 low wavenumber peaks testifies the presence of large-scale structures akin to Langmuir
 897 turbulence as theoretically derived by Huang & Mei (2006) and experimentally observed
 898 by Nepf & Monismith (1991). It was also brought up a parallelism between Langmuir
 899 turbulence, where the current is generated by the wind blowing on the free-surface and
 900 our situation, where the current is generated by the presence of a pressure gradient.
 901 In the latter case, a slightly modified turbulent Langmuir number La_t was introduced
 902 to effectively discern when Langmuir-type cells start to populate the wave-dominated
 903 region of the flow. Based on our data, the threshold was determined as $La_t < 4 - 5.53$
 904 (that is equivalent to the imposition of two conditions, i.e. $af_w/u_{\tau_c} > 0.26 - 0.5$ and
 905 $\epsilon > 0.02$). Ongoing work by the authors is currently dedicated to experimentally verify
 906 this proposed pictorial view of turbulence in combined wave-current flows.

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