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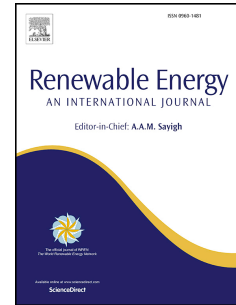
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# Energy and mechanical aspects on the thermal activation of diaphragm walls for heating and cooling

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## Abstract

Underground geotechnical structures, such as deep and shallow foundations, diaphragm walls, tunnel linings and anchors are being increasingly employed as energy geostructures to exchange heat with the ground by installing absorber pipes into the structural elements. This paper focuses on the application of this technology to reinforced concrete diaphragm walls used for construction of underground car parks, basements and metro stations, with the purpose of heating and cooling the adjacent buildings. Preliminary numerical modelling allowed optimising the geothermal plant design of the diaphragm wall. Then its energy efficiency is investigated through finite element thermo-hydro coupled analyses together with the effects of the thermal activation on the surrounding soil. Finally, finite difference thermo-mechanical analyses are used to study the mechanical effects induced by the thermal activation.

**Keywords:** energy walls, energy geostructures, thermo-hydraulic numerical analyses, thermo-mechanical numerical analyses, shallow geothermal energy

## 1. Introduction

Underground geotechnical structures, such as deep and shallow foundations, diaphragm walls, tunnel linings and anchors are being increasingly employed as energy geostructures, in Europe and all around the world [1,2]. Research studies on this technology are increasingly being carried out, including investigations on their energy efficiency and geotechnical behaviour [3–7]. Besides being constructed for their primary structural role, they are equipped to be able to exchange heat with the ground and supply thermal energy for heating and cooling of buildings and de-icing of infrastructures. This technology can play a fundamental role in the current challenge of facing the increasing need for clean and renewable sources of energy. The thermal activation is achieved by installing absorber pipes in the geostructures, in which a circulating fluid extracts or injects heat from or into the ground. The polyethylene Pe-Xa pipes are usually attached to the reinforcing steel cage, before concrete cast. The pipes inlet/outlet of each panel are connected to the main circuit, which links them to the heat pumps.

These systems belong to the category of low enthalpy geothermal plants and are associated with heat pumps. A number of real applications of this technology are already operational especially in Austria, Germany, United Kingdom and Switzerland [8–15].

This paper focuses mainly on the application of this technology to the diaphragm walls of underground car parks, basements and metro stations, with the purpose of using them for heating and cooling the adjacent buildings. Efforts have already been devoted to study similar applications [4,16–19] and a number of real operational systems exist [20–22].

The main goals of this study were to optimise the heat exchanger pipe circuit in the walls, quantify the extractable and injectable heat, and study the consequences in terms of structural behaviour and effects on the surrounding ground.

A reference diaphragm wall, with the geometry shown in Figure 1 was considered in all the simulations presented in this paper, i.e. a wall panel 2.5 m wide, 0.8 m thick and 15.5 m deep. The absorber pipes are installed only on the ground side. Another solution would have been to have a double circuit, one on each

46 side, in order to foster the heat exchange with the air inside the excavation. The efficiency of this solution  
47 needs to be checked.

48 Preliminary numerical analyses were conducted to assess the optimal geothermal plant design for the  
49 diaphragm wall. Then, three main aspects were investigated with reference to the optimal setting: the energy  
50 performance, the temperature variation in the surrounding soil and the mechanical effects induced on the  
51 wall by its thermal activation.

## 52 **2. Energy performance and thermal aspects**

53 With the aim to assess the energy efficiency of thermo-active walls analysing the thermal exchange between  
54 the fluid circulating through the pipes, the concrete structure and the surrounding saturated soil, thermo-  
55 hydro simulations were performed through the finite element software FEFLOW© [23]. The procedure  
56 adopted is consistent to that described in [24] for energy tunnels.

57 The thermo-hydro problem is governed by the mass conservation equation, the energy conservation equation  
58 and the Darcy's velocity law.

59 The absorber pipes were reproduced through the 1D discrete features elements provided in the software. The  
60 use of these elements to simulate pipes in geothermal systems has been validated and showed good  
61 agreement when compared to analytical solutions [23]. In this study, the pipes are considered to have a  
62 diameter of 25 mm and thickness of 2.3 mm. The material properties used in the simulations are listed in  
63 Table 1. Parameters related to the ground are representative of the Torino (Italy) subsoil, according to  
64 previous studies where specific pumping tests were executed [25].

### 65 66 **2.1. Study on the pipe configuration and fluid velocity**

67 The geometry of the pipe circuit is a compromise between the construction feasibility and the optimised heat  
68 exchange.

69 Simplified models were built in order to identify the most efficient configuration in terms of heat exchange,  
70 depending on the position of the pipes and the fluid velocity. The initial temperature was set equal to 14 °C  
71 for the whole domain, while the ground water level was set to 5 m below the ground surface. For the sake of  
72 simplicity, no ground water flow was considered in these preliminary analyses. The bottom and far field  
73 boundaries (left side in Figure 2a) were checked to be far enough not to influence the results. External air  
74 temperature was imposed on the top boundary, considering the seasonal fluctuation in the city of Torino  
75 (based on data from [26]), representative of a temperate humid European climate, according to Figure 2b.  
76 The choice of the wall thermal boundary condition (interior of the tunnel or car park or basement) is one of  
77 the most peculiar aspects of energy walls or tunnels modelling, as it represents a major difference with  
78 respect to more conventional borehole heat exchangers or energy piles, which are surrounded by soil  
79 [3,6,27]. However, for these preliminary analyses which have the purpose to find the most efficient solution  
80 and not to provide reliable data of exchanged heat, adiabatic condition was imposed on the right side of the  
81 model. For the thermal-hydraulic analysis, the model was firstly run for one year without activation of the  
82 geothermal plant to initialise the soil temperature according to the external air variation. The plant was then  
83 activated by injecting water into the pipes at a constant inlet temperature of 4°C for one month (Figure 2b),  
84 reproducing the heat extraction condition (winter mode). Three pipe configurations were considered, as  
85 illustrated in Figure 3. It has to be noticed that the pipes length is different for the three configurations, i.e. it  
86 is 55.5 m for case (a), 68.0 m for case (b) and 50.0 m for case (c). For the three configurations, the analyses  
87 were repeated for different values of fluid velocity, between 0.1 and 1.0 m/s. The exchanged heat, expressed  
88 in Watt was computed as:

$$Q = mc_w |T_{wo} - T_{wi}| \quad (1)$$

89 where  $m$  is the mass flow rate inside the pipes expressed in kg/s,  $T_{wo}$  the fluid outlet temperature and  $T_{wi}$  the  
90 imposed fluid inlet temperature.

91 The results obtained for the three pipe configurations and imposing a fluid velocity of 0.6 m/s are reported in  
92 Table 2. The exchanged heat per linear meter,  $Q_l$  is intended per meter of pipe, while that per square meter,  
93  $Q_s$ , is intended per wall surface area. Configuration (c) appears to be the most favourable. This result is in  
94 agreement with the outcome of a similar study carried out by [19], although other studies highlighted  
95 concerns about the practical applicability of such horizontal configuration [20]. Because of the amount of  
96 reinforcement usually used in diaphragm walls, placing pipes horizontally might be more difficult and it may  
97 affect the way in which the concrete spreads in the trench to more extent than the vertical placement.  
98 Figure 4 shows the influence of the fluid velocity on the exchanged heat for the pipes configuration (c). All  
99 considered velocities allow for a turbulent flow to occur into the pipes. Differently from the case of energy  
100 tunnels [3], the fluid velocity does not affect the exchanged heat in a significant manner. However, this was  
101 also confirmed by other similar studies on energy piles and walls [4,19,28]. Considering these results and the  
102 mentioned difference in pipes length, configuration (c) was chosen as the optimised one in the following,  
103 assuming a fluid velocity of 0.2 m/s.

## 104 2.2. Quantification of heat exchange potential

106 A more advanced 3D numerical model was built to properly quantify the exchangeable heat. The model  
107 reproduces with more details the geometry of the diaphragm wall by including a 1.0 m thick concrete slab  
108 which constitutes the base of the excavation, at 9.5 m below the ground surface (Figure 5a). The surrounding  
109 ground is also included with the dimensions shown in the same figure. The bottom, left and right boundaries  
110 were checked to be far enough not to influence the results. The same external air temperature seasonal  
111 variation was adopted and imposed to the top boundary. The assumption on how to model the internal air  
112 was previously studied by [27] and the comparison between different boundary conditions showed that  
113 modelling the air is equivalent to consider the wall as adiabatic and was adopted in the following. Air was  
114 thus modelled through finite elements, having air thermal properties. This assumption is the most  
115 conservative in terms of energy efficiency, because the heat exchange between the wall and the air is  
116 neglected. Two groundwater flow conditions were considered: (i) no flow and (b) ground water flow velocity  
117 of 1.5 m/d. Based on the previous analysis, the inlet fluid velocity was assumed equal to 0.2 m/s.

118 The time frame in the analysis is 4 years. The first year reproduces the situation before the activation of the  
119 geothermal plant, where only the external air temperature is present. This is followed by three years of  
120 heating-cooling mode. A double mode usage of the geothermal plant was considered, including three months  
121 heating and three months cooling, according to the input presented in Figure 5b. It is to be mentioned that  
122 this condition does not fully reproduce the proper behaviour of a real heat pump system where the  
123 temperature of the fluid would be the result of the use of the heat pump and, generally, will not remain  
124 constant over a season. Although real systems operate within varying and complex energy demand (and  
125 hence inlet temperature) patterns, a constant temperature has been used in the following to provide (a)  
126 a simpler and controlled conditions comparison of the parameters under consideration and (b) a more generally  
127 applicable approach within the wide range of thermal demand scenarios relevant to different building  
128 typologies.

129 The obtained results are illustrated in Figure 6a and 6b, in terms of inlet/outlet temperature and exchanged  
130 heat per meter of wall depth, respectively. It can be concluded that the heat exchange is highly affected by  
131 the presence of the ground water flow, as confirmed by similar studies [29]. The peak and end of cycle  
132 values of exchanged heat in winter and summer conditions for the two-considered ground water flow  
133 velocities are presented in Table 3 and Table 4. It has to be noticed that these results are valid for the  
134 considered geometry and conditions, i.e. for the upstream side of the structure.

135 To quantify the effective benefit in using this technology, the obtained results in terms of heat exchange were  
136 compared with the average energy demand of a typical apartment building. Data on energy consumption for  
137 heating were extracted from the database delivered by the European project Tabula [30], which provides an  
138 overall figure of energy demand in Europe for several typical buildings. The apartment block building

139 typology for a middle climatic zone (Italy) was selected from the database. As the energy demand seems to  
 140 be highly influenced by the construction period due to the continuum innovation in the refurbishment  
 141 process, two construction periods were considered, i.e. early 1900 and after 2006. Two different heat supply  
 142 systems correspond to the two selected periods, both based on a gas central heating system. A non-  
 143 condensing boiler (atmospheric burner), central distribution and gas-fired instantaneous water heater was  
 144 considered for the 1900 constructions while a low temperature standard boiler and gas-fired instantaneous  
 145 water heater with condensing plus thermal solar plant was considered for most recent constructions.  
 146 Assuming to turn on the geothermal system for 1800 hours per year for heating during winter, the number of  
 147 energy wall panels needed for heating was computed. Table 5 illustrates the results of this analysis for both  
 148 the considered groundwater flow conditions. The number of panels needed for apartment buildings are in  
 149 line with the geometry that an underground car park for the residents' use would likely have. This is  
 150 particularly true with the today's figures in terms of energy needs, showing that application of energy  
 151 geostructures is promising if considered from the design stage of new construction.

### 152 2.3. Thermal effects on the surrounding soil

153 In order to evaluate the influence on the ground temperature, Figure 7 shows its evolution 5 m far from the  
 154 wall contour, at different depths in the case of no flow and of groundwater flow of 1.5 m/d. Up to about 5.5  
 155 m depth (point A in Figure 5) the ground temperature is affected by the external air temperature fluctuation  
 156 (see first year results). The activation of the geothermal system induces a variation of temperature in both  
 157 points A and B of +/- 1.5 °C with respect to the temperature variation experienced during the first year of  
 158 simulation.

## 159 3. Thermo-mechanical aspects

160 When the diaphragm wall is used as ground heat exchanger, it is subjected to seasonally cyclic temperature  
 161 variations. This could have different consequences: on the one hand, the wall could undergo thermal  
 162 deformation according to its constitutive behaviour, on the other hand, its thermal deformation could be  
 163 partially prevented by the surrounding soil and possible over structure thus inducing additional stresses. As  
 164 the primary structural role of the retaining structure must always be ensured, 2D plane strain thermo-  
 165 mechanical analyses were conducted to investigate the thermal-induced mechanical effects on the diaphragm  
 166 wall caused by its thermal activation. The Finite Difference Method (FDM) software FLAC [31] was used  
 167 for this purpose.

### 168 3.1. Mathematical formulation

169 The thermal option of FLAC incorporates both conduction and advection models, but only conduction was  
 170 considered in the following analyses. The heat transfer and the consequent temperature changes are coupled  
 171 to the mechanical calculations at any time during a transient simulation. The coupling occurs in one direction  
 172 only, i.e., the temperature may result in stress changes, but mechanical changes in the body resulting from  
 173 force application do not result in temperature change. This restriction is not believed to be of great  
 174 significance here, since the energy changes for quasi-static mechanical problems are usually negligible. Both  
 175 soil and concrete are assumed to behave thermo-elastically and consequently their thermal deformation  $\varepsilon_{ij}^T$   
 176 is computed as:

$$\frac{\partial \varepsilon_{ij}^T}{\partial t} = \beta \frac{\partial T}{\partial t} \delta_{ij} \quad (2)$$

177 where  $t$  is the time,  $T$  is the temperature,  $\beta$  is the coefficient of linear thermal expansion [ $1/^\circ\text{C}$ ] and  $\delta_{ij}$  is the  
 178 Kronecker delta. From this, constitutive equations are reformulated to solve thermal stress problems [31].  
 179 The differential equation of motion and the rate of strain-velocity relations used in the FDM are based upon  
 180 mechanical considerations and are unchanged for thermo-mechanics.

### 181 3.2. Structural behaviour

182 Numerical analyses were performed with reference to the geometry of the reference diaphragm wall and  
 183 positioning of the geothermal pipes adopted for thermo-hydraulic analyses. The mesh characteristics and



184 dimensions (14880 quadrilateral elements) are shown in Figure 8 together with the boundary conditions  
185 adopted. Plane strain conditions are assumed to hold true. Particular care was adopted to include the correct  
186 size of the pipes in the mesh as shown in the same Figure 8. Thermal parameters adopted are the ones given  
187 in Table 1, while mechanical parameters were chosen based on previous studies [32] and are listed in Table  
188 6. Elastic perfectly plastic behaviour was assumed for the soil while a linear elastic behaviour for concrete.

189 The numerical analysis is performed in different stages. A first group of stages is used to assess the stress  
190 state in the ground resulting from the construction of the diaphragm wall and the base slab, before the  
191 geothermal system is activated. Only mechanical loading is applied during these stages. The construction of  
192 the diaphragm wall is simulated by applying appropriate mechanical parameters to the finite elements in the  
193 corresponding wall area. The excavation is then performed in three subsequent stages (two first layers of 3.2  
194 m and a third layer of 3.1 m), obtaining a final excavation depth of 9.5 m. The base slab is afterwards  
195 installed by applying the mechanical properties of concrete to the corresponding finite elements. For the sake  
196 of simplicity, the seepage problem arising by excavating below the ground water table is not considered in  
197 the computation (TM simulations).

198 A second group of stages simulates the operation of the geothermal system. For these stages, the thermo-  
199 mechanical coupling is activated. Thermal boundary conditions in the model are consistent to those adopted  
200 for the thermo-hydraulic analysis previously described. The internal boundary on the car park side is  
201 assumed with a fixed temperature of 14°C, considered as the mean value year-round. The model is firstly  
202 subjected to the variation of the external air temperature only, for the period of 3 years (starting from 1<sup>st</sup>  
203 January). The external air temperature is the same as that adopted for thermo-hydraulic analyses and shown  
204 in Figure 5b. Then, the operation of the geothermal system is simulated for a period of 1000 additional days  
205 (i.e. 2 years and 270 days). This is done by applying an input temperature to the nodes of the elements which  
206 reproduce the pipes contour. The input temperature is varied along the year, following the heating and  
207 cooling cycles, by applying the corresponding values determined from the thermo-hydraulic analyses,  
208 averaged over the pipes length.

209 Figure 9, shows the temperature distribution in the wall and ground with colour scale labels, for two  
210 moments, representative of the summer (August) and winter (February). This is shown to be in fair  
211 agreement with the data from thermo-hydraulic analyses of Figure 7. However, some time shift is present,  
212 due to the different assumption for thermal flow and for the boundary condition adopted at the wall internal  
213 side between the thermo-hydraulic and the thermo-mechanical analyses. Also, advection is not considered in  
214 the TM analyses. While it plays a major role when assessing the temperature distribution in the ground (it  
215 was included in fact in the TH analyses), it does not influence much the temperature distribution within the  
216 wall itself. The comparison between the computed temperatures (Figs. 7 and 9) is considered in sufficient  
217 agreement to justify neglecting advection.

218 To evaluate the thermal induced stress strain behaviour of the wall, Figure 10 shows the horizontal  
219 displacement at the top of the diaphragm wall and the temperature variations, computed for some selected  
220 measuring points located at different depths from the top of the diaphragm wall (at 3.5 m, 9.5 m and 16 m).  
221 Selected measuring points located at the same depth (9.5 m) but at different distances from the diaphragm  
222 wall (on the pipes axis, 1.2 m and 3.2 m on the ground side) are shown instead in Figure 11. Both figures  
223 show data pertaining to the stages when the geothermal system is in operation; starting values in the plots  
224 correspond to the results of the construction stages.

225 It is shown that during the first three years, the temperature change is strictly dependent from the external air  
226 temperature for point A, while points B, C, D and E are ranging around a constant value due to their depth.  
227 Moreover, some deviation with time occurs to the horizontal displacement at the top of the diaphragm wall.  
228 Two full year cycles are needed before the seasonal fluctuation reaches equilibrium (this was verified by  
229 running a separate thermo-mechanical analysis where only the external air temperature effect is considered  
230 up to four years cycle).

231 When the geothermal system is activated, the temperature change in the points is strictly dependent from the  
232 input temperature in the pipes. This influence is reduced for point C, in the ground below the diaphragm  
233 wall. Again, an initial deviation with time of the stresses and displacements computed is experienced and

234 becomes negligible only after the first two years of activation. Temperature changes of points B, D and E are  
235 shown to slowly decrease while moving further from the pipes, also, the temperature peaks are shifted in  
236 time due to the thermal inertia of the soil.

237 Thermal cycles in the pipes cause deformation of the original wall deflection. Figures 12a and 12c show the  
238 horizontal displacements computed along the diaphragm wall vertical axis. Maximum displacement variation  
239 with respect to undisturbed state is shown for that pertaining to summer operation mode (August, Fig. 12a)  
240 and to winter operation mode (February, Fig. 12c). Also shown in the same figures are the displacements  
241 computed during mechanical loading. As the top wall deformation is not restricted, the deflection due to  
242 thermal cycles is quite evident, ranging between 16 to 22 mm (the maximum values are obtained during  
243 summer). This corresponds to a 16% displacement increase during summer and a 4% decrease during winter  
244 induced by the thermal activation of the geothermal system. The variation in terms of bending moment  
245 during heating and cooling cycles, is shown in Figures 12b and 12d. At the base of the wall, the presence of  
246 the base slab and the ground creates constraints to wall movements. Thermal induced horizontal  
247 displacements are negligible, while the corresponding stress change rises up producing a maximum bending  
248 moment of 460 kNm/m during winter (i.e. an increase of 17%). Bending moment is minimum during  
249 Summer, when the displacement is maximum, and reaches highest values in winter. The stress variations  
250 computed in the analyses are largely below the strength limits of the structure.

251 Similar analyses conducted by [6] have shown minor displacements at the head as an effect of thermal  
252 activation (less than 10 mm). However, in the mentioned study, despite minor variation in parameters and  
253 geometry, a major difference lied in the fact that the diaphragm wall was subjected to different constraints. A  
254 top slab was present in [6] study, whereas here the diaphragm wall is considered unrestrained at the top, to  
255 investigate a different restraint configuration. An additional numerical analysis was carried out by  
256 constraining the top of the diaphragm wall by a roller (i.e. similar conditions to the [6] study). The results  
257 confirmed lower displacements and bending moments along the diaphragm wall with respect to the non-  
258 constrained configuration. The latest, presented in this paper, can therefore be considered a conservative  
259 condition. Moreover, this is a clear (and rather obvious) indication that the mechanical effects of thermal  
260 loading on diaphragm wall are strictly function of the structural conditions. This implies that the proper  
261 construction sequence and structural behaviour need to be properly simulated in the numerical analysis to  
262 allow for reliable results to be obtained.

263

#### 264 4. Conclusions

265 Based on the results of the computations described, one can draw the following conclusions:

- 266 • The horizontal configuration of pipes geometry allows maximising the heat exchange.
- 267 • The energy wall system for the conditions considered in this paper would allow to exchange between  
268 20 to 25 W per square meter of wall, in Winter and in Summer respectively, when the ground water  
269 is static, that rises up to 40 to 50 W/m<sup>2</sup> when a favourable underground water flow exist.
- 270 • These figures would allow to cover energy needs of a typical apartment building, imaging to  
271 thermally activate an underground car park serving the building.
- 272 • The influence on the surrounding ground in terms of temperature variation of the groundwater flow  
273 is within 7°C at 5 m distance from the wall boundary with full recovery after the year-round cycle  
274 starting from the second year of operation.
- 275 • Bending moment and horizontal displacement at the top of the diaphragm wall increase up to 16%  
276 due to thermal activation. However, stress variations computed are largely compatible with strength  
277 limits.

278 The above findings are in line with research results and case study data previously published and underline  
279 the interest to improve the understanding of the geothermal process to enhance the potential use of  
280 underground infrastructures as effective and innovative heat exchangers for the future.

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360

Table 1 – Material properties (Barla et al., 2013).

Property	Heat carrier fluid	Ground	Concrete
Horizontal hydraulic conductivity, $k_x=k_z$ [m/s]	-	$4.15 \cdot 10^{-3}$	$10^{-16}$
Vertical hydraulic conductivity, $k_y$ [m/s]	-	$2.075 \cdot 10^{-4}$	$10^{-16}$
Specific storage coefficient, $S$ [ $m^{-1}$ ]	-	$10^{-4}$	$10^{-4}$
Porosity, $n$ [-]	-	$0.25 \div 0.3$	0.0
Heat capacity, $\rho c$ [ $MJ/m^3/K$ ]	4.2	2.55	2.19
Thermal conductivity, $\lambda$ [ $W/m/K$ ]	0.65	$2.26 \div 2.8$	2.3
Longitudinal dispersivity, $\alpha_L$ [m]	-	3.1	-
Transverse dispersivity, $\alpha_T$ [m]	-	0.3	-

Table 2 – Extracted heat in the different configurations considered (fluid velocity = 0.6 m/s).

	$Q$ [W]	$Q_l$ [W/m]	$Q_s$ [W/m <sup>2</sup> ]
(a) VP	290.11	5.22	7.49
(b) HP - $i=0.5m$	324.09	4.76	8.36
(c) HP - $i=1.0m$	313.71	6.26	8.10

Table 3 – Extracted and injected heat with no ground water flow.

	Peak	End of cycle	Peak	End of cycle
	W/m	W/m	W/m <sup>2</sup>	W/m <sup>2</sup>
<b>Winter</b>	51.0	17.2	20.4	6.9
<b>Summer</b>	63.0	24.0	25.2	9.6

Table 4 – Extracted and injected heat with a ground water flow velocity of 1.5 m/day.

	Peak	Steady state	Peak	Steady state
	W/m	W/m	W/m <sup>2</sup>	W/m <sup>2</sup>
<b>Winter</b>	98.8	70.4	39.5	28.1
<b>Summer</b>	123.5	90.4	49.4	36.2

Table 5 – Examples of possible applications for a common building typology: apartment block (data on energy demand from Corrado et al., 2012).

Construction period	Total Energy need [MWh]	Exchanged heat per wall panel [kW]		Operation duration [h]	Exchanged heat per wall panel [kWh]		Number of needed panels	
		gwf=0 m/d	gwf=1.5 m/d		gwf=0 m/d	gwf=1.5 m/d	gwf=0 m/d	gwf=1.5 m/d
Today	113.5	0.27	1.1	1800.0	486.0	1962.0	233	58
1900	270.5						557	138

Table 6 – Geotechnical parameters adopted for the thermo-mechanical numerical analysis.

Property	Ground	Concrete
Unit weight [kN/m <sup>3</sup> ]	19.5	25
Elastic modulus [MPa]	215	33300
Poisson's ratio [-]	0.3	0.2
Bulk modulus [MPa]	179	18500
Shear modulus [MPa]	83	13875
Cohesion [kPa]	15	-
Friction angle [°]	38	-
Thermal conductivity, $\lambda$ [W/m/K]	2.8	2.3
Linear thermal expansion, $\beta$ [1/°C]	$10^{-5}$	$1.2 \cdot 10^{-5}$
Specific heat [J/kg°C]	1053	876

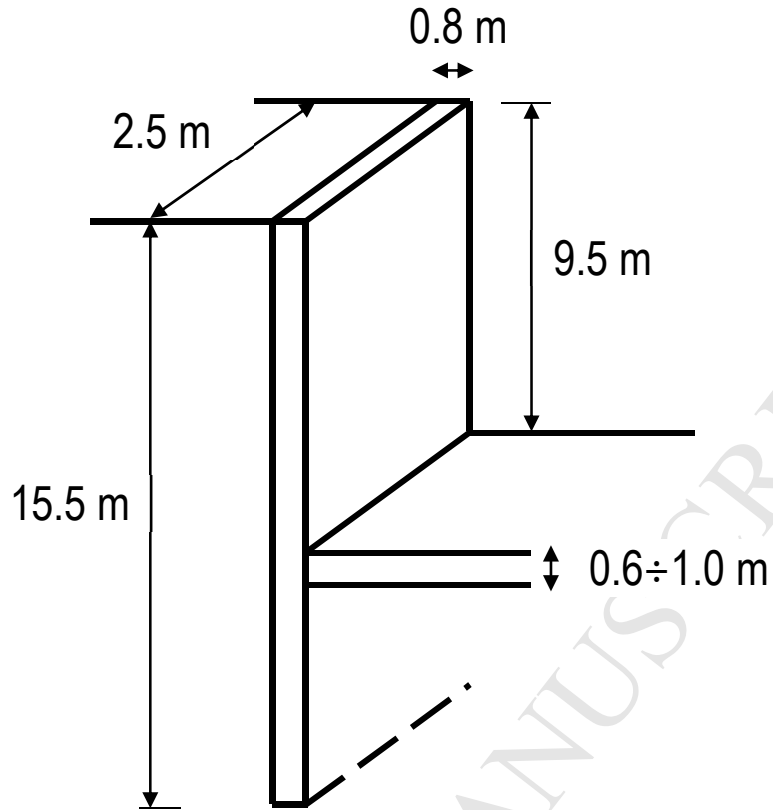


Figure 1- Geometry of the reference energy wall panel.

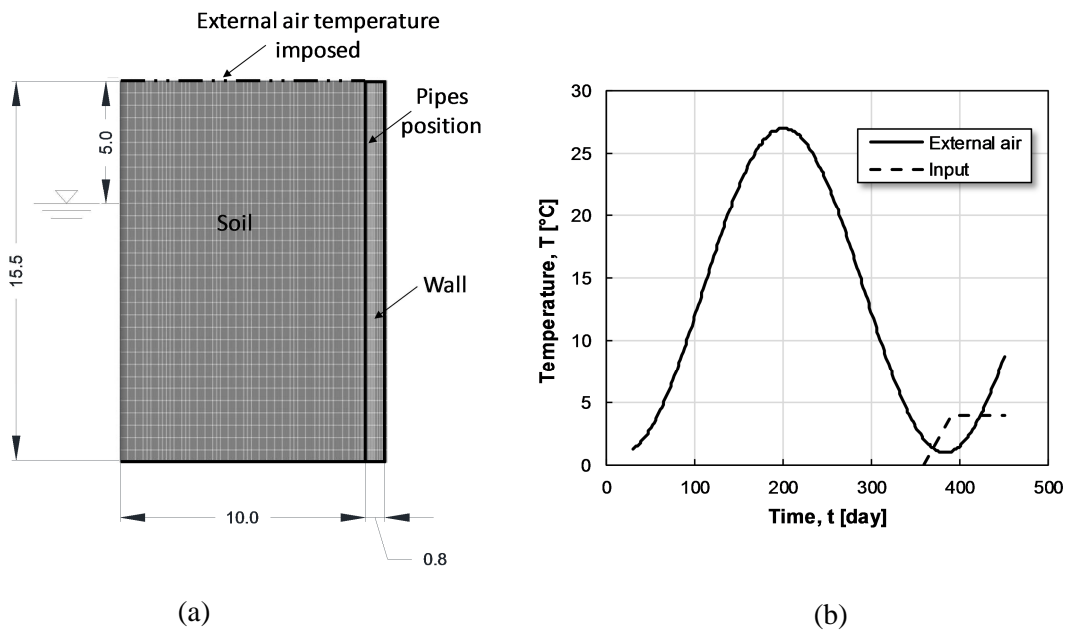


Figure 2 – (a) Geometry and boundary conditions of the FE model and (b) external air and inlet temperature.

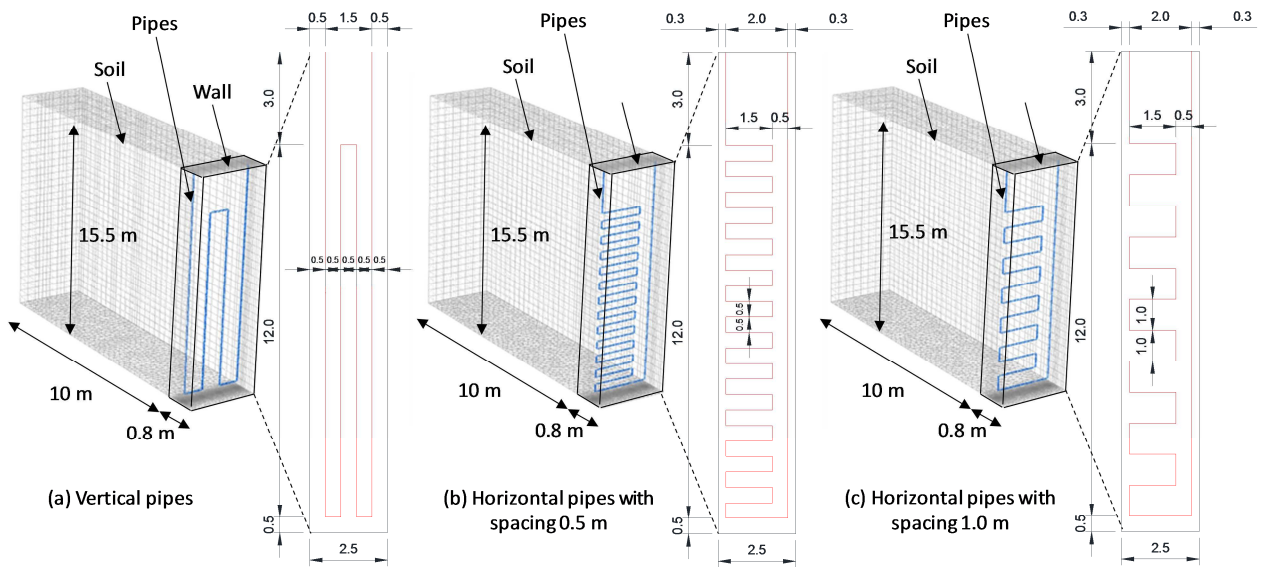


Figure 3 - Pipes configurations.

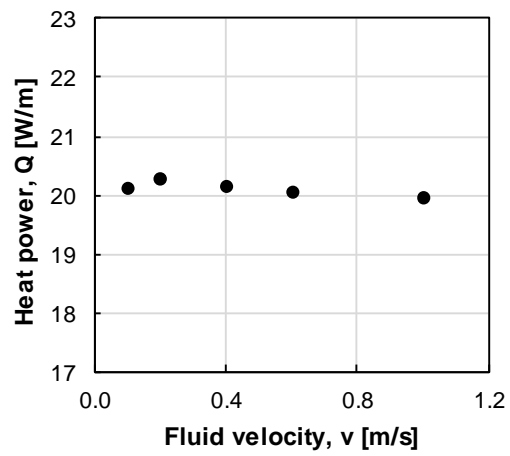


Figure 4 – Extracted heat as a function of the heat carrier fluid velocity in the pipes.



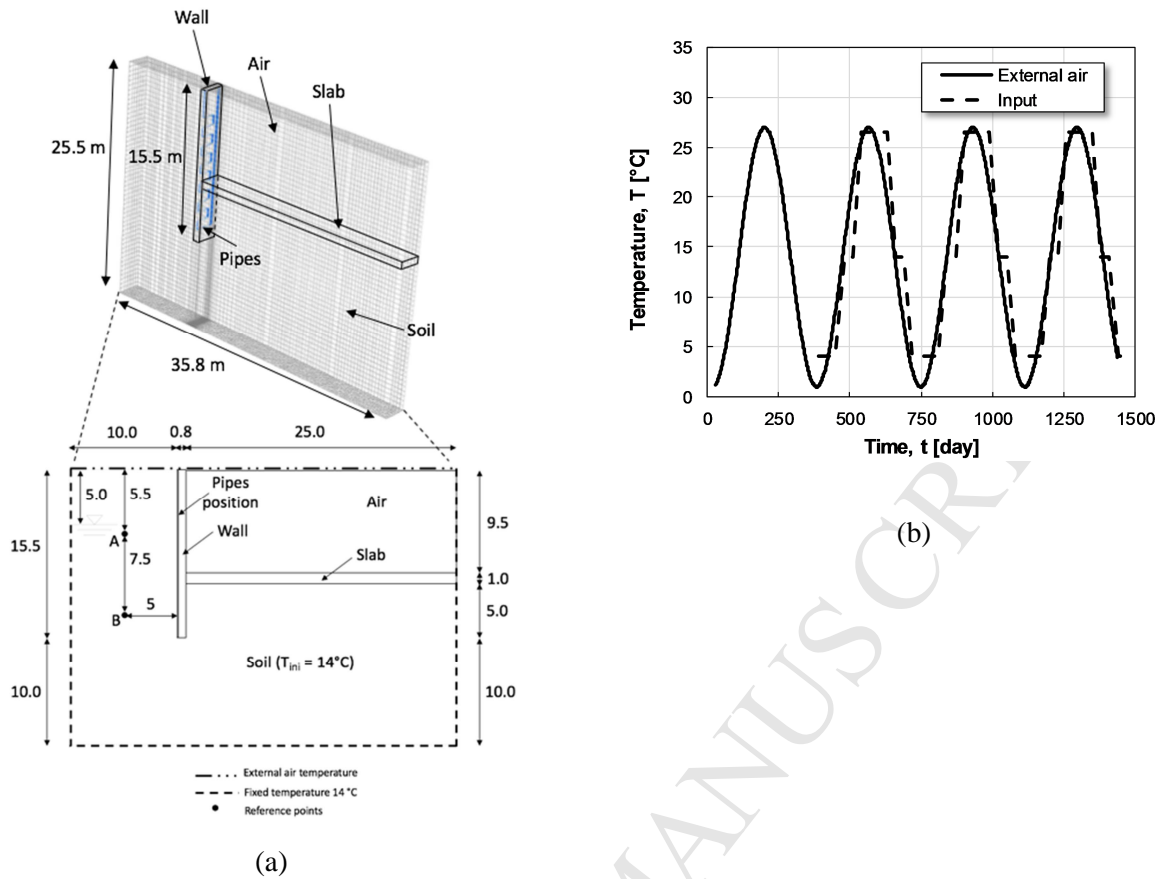


Figure 5 – (a) Geometry of the model and (b) imposed cyclic inlet temperature and external air temperature variation in Torino.

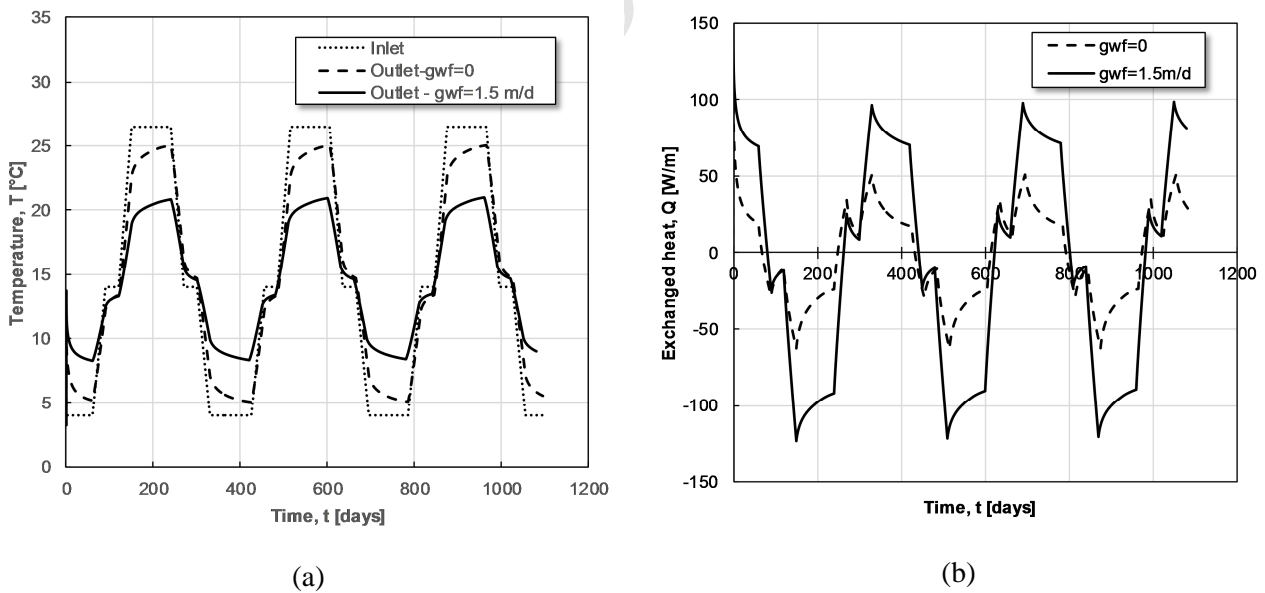


Figure 6 – (a) Inlet and outlet temperature in the pipes and (b) exchanged heat during three years' simulation, in the case of 0 and 1.5 m/day of ground water flow.

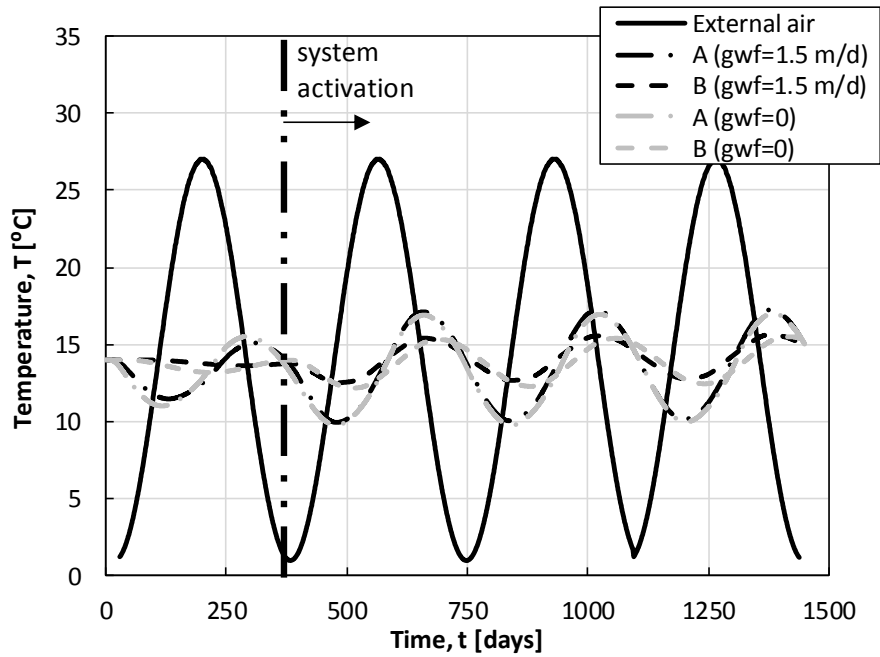


Figure 7 – Temperature variation in the ground 5 m far from the wall at different depths, in the case of 0 and 1.5 m/day of ground water velocity.

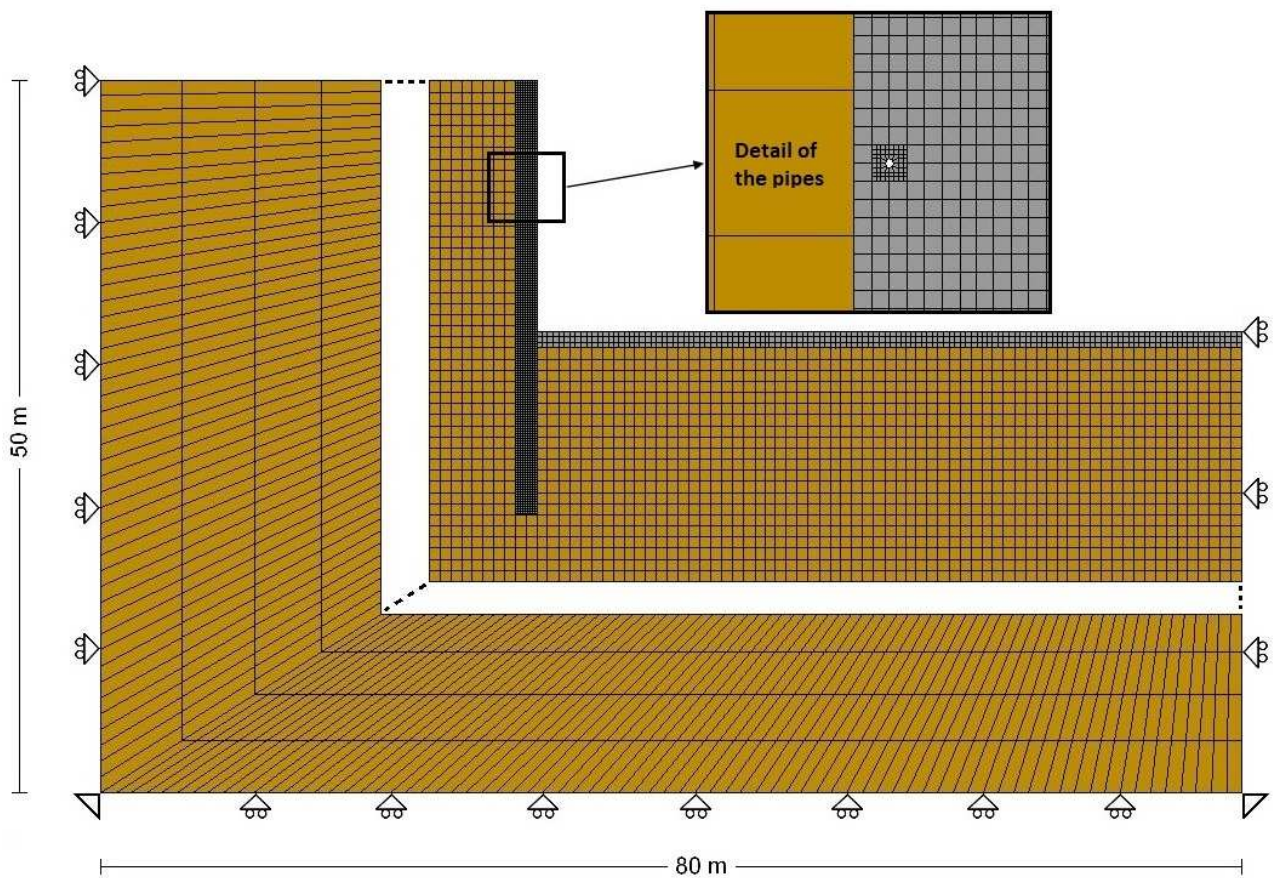


Figure 8 – Finite difference mesh adopted for thermo-mechanical analyses: model, size, boundary conditions and detail of the mesh in the pipe area.

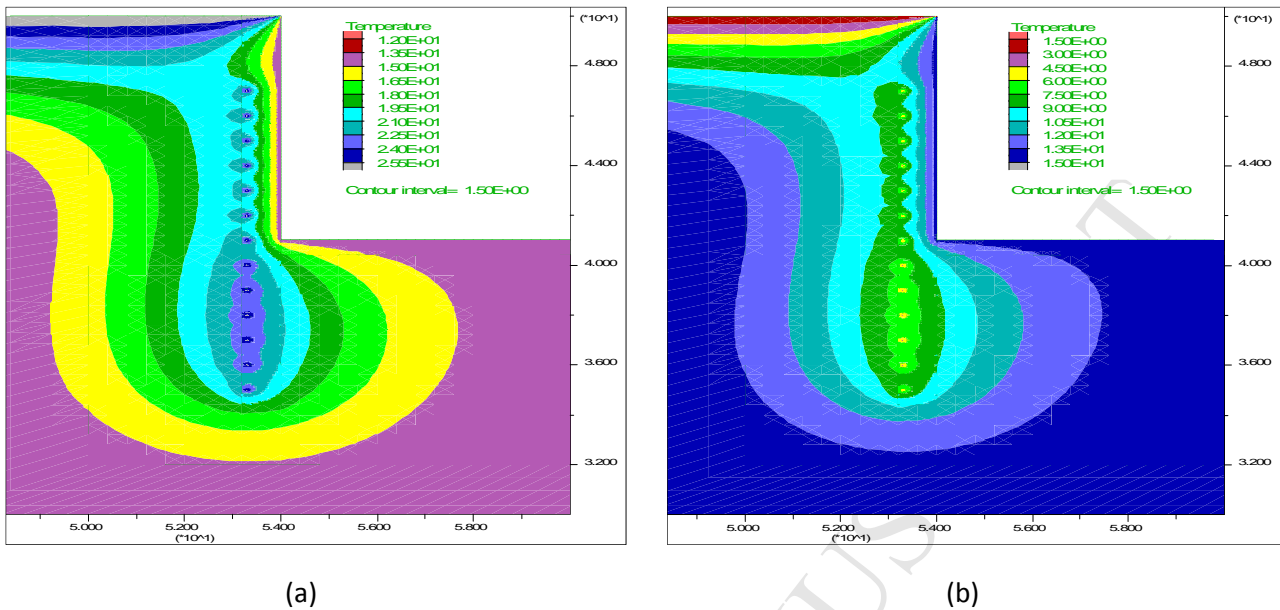


Figure 9 – Temperature distribution in °C in the ground in August (a) and February (b) with the geothermal system activated.

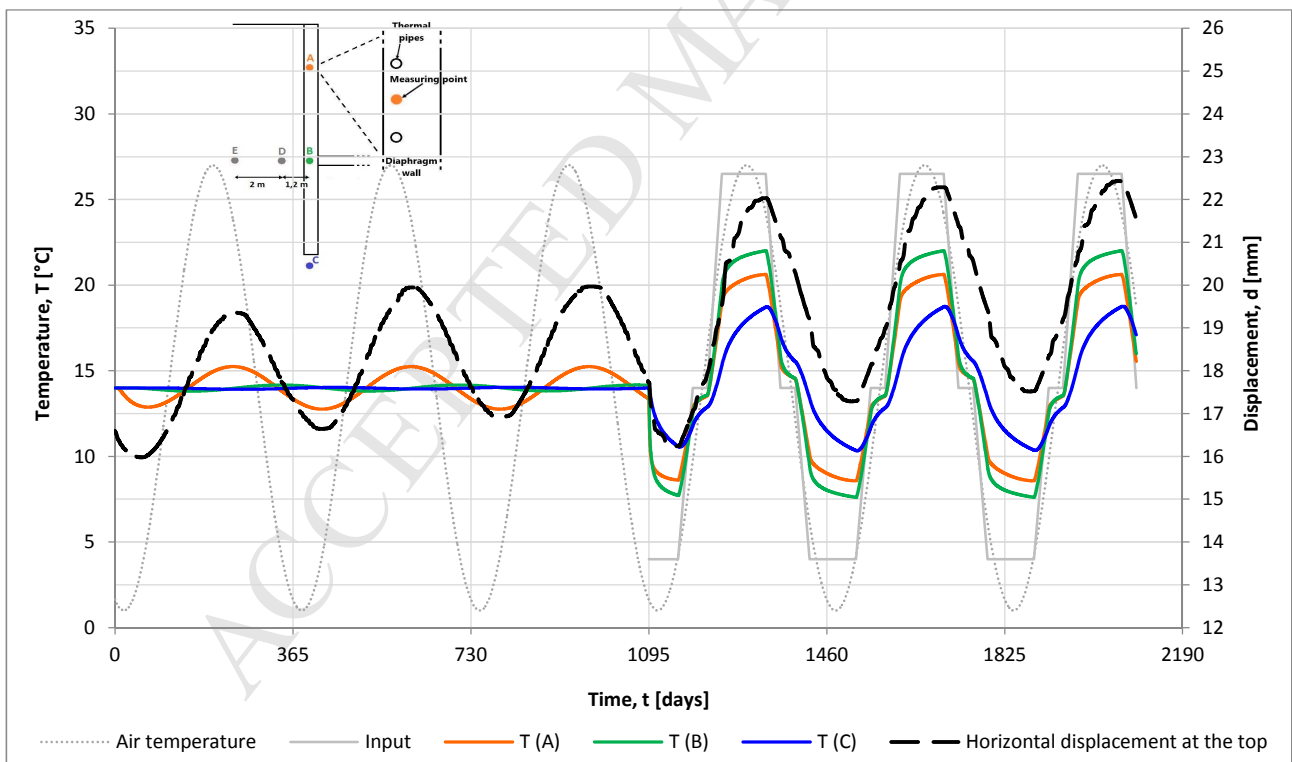


Figure 10 – Displacement at the top of the diaphragm wall, input temperature variation in the pipes and external air temperature assumed during computation compared to that computed at specific measuring points (A, B, C).

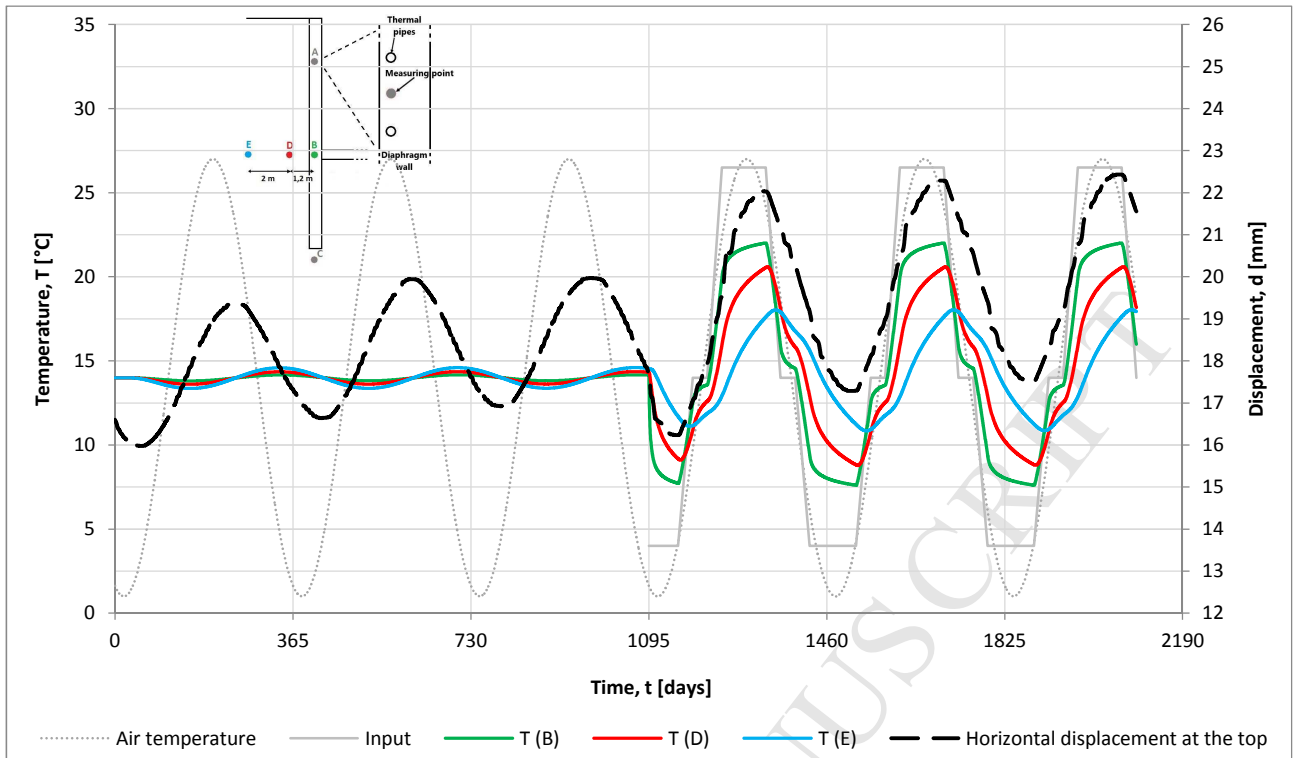


Figure 11 – Displacement at the top of the diaphragm wall, input temperature variation in the pipes and external air temperature assumed during computation compared to that computed at specific measuring points (B, D, E).

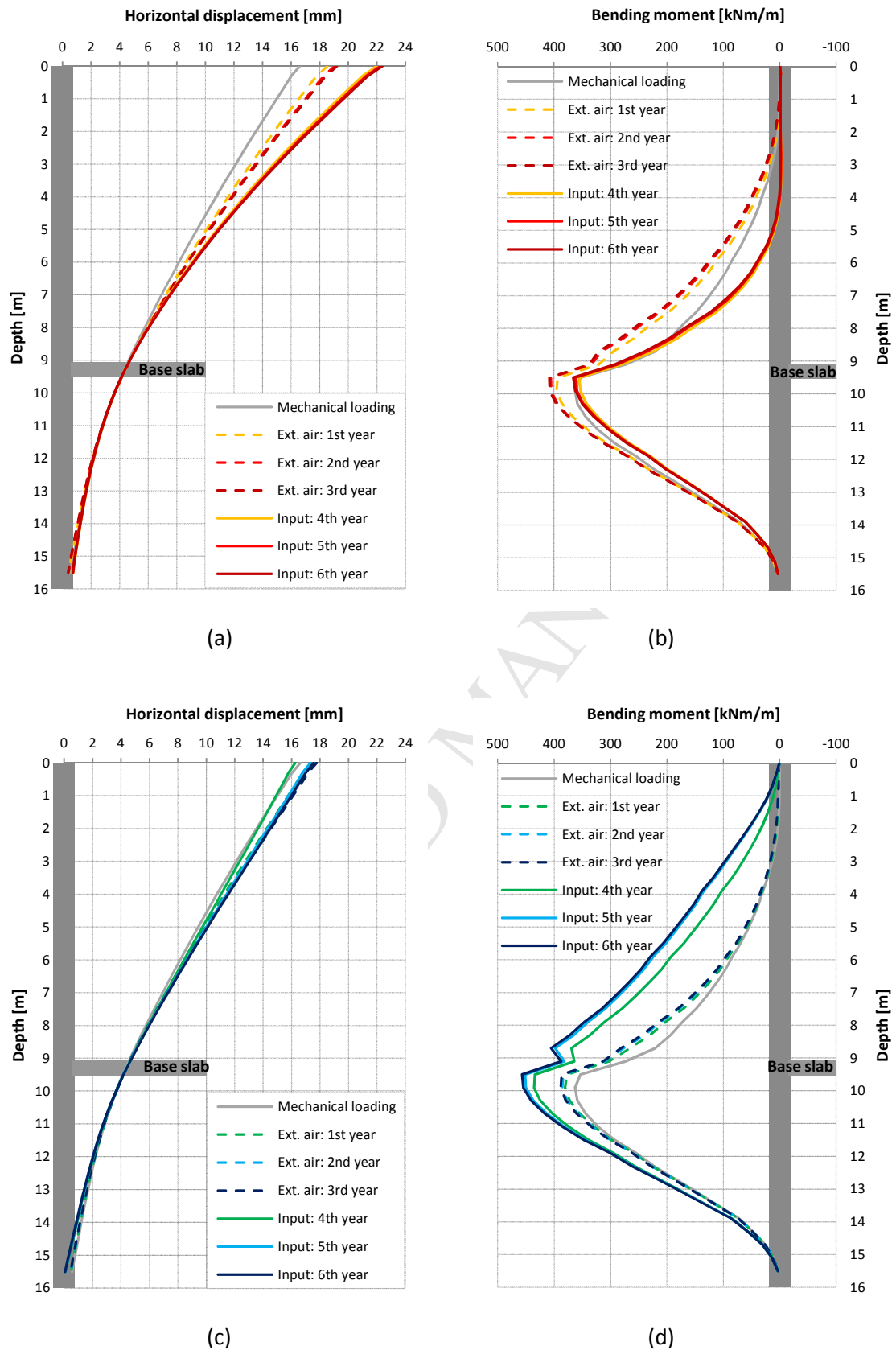


Figure 12 – Bending moment and horizontal displacement change with: mechanical loading, external air and the activation of the geothermal system in Summer (August, (a), (b)) and Winter (February, (c), (d)).

- Thermal and mechanical aspects of energy walls are discussed.
- The horizontal configuration of pipes geometry allows maximising the heat exchange.
- Heat exchange between 20 to 25 W/m<sup>2</sup> with static ground water.
- Heat exchange between 40 to 50 W/m<sup>2</sup> with favourable groundwater flow.
- Bending moment increase up to 16% due to thermal activation.

ACCEPTED MANUSCRIPT