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Comparative Assessment of Injection Strategies for Highly Concentrated Nano Fe/Cu Particles into Sand Columns

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Abstract

The transport of bimetallic nano-*Fe/Cu* particles through coarse sand-packed columns was investigated simulating particle transport under 25 injection strategy scenarios. The considered transport mechanisms included retention on and release from the solid grains, modeled by a dual-site advection-dispersion-deposition equation, and clogging of the porous medium. The transport kinetics and parameters used in this study were calibrated against experimental data, previously reported, and simulated using E-MNM1D. The influence of the injected particle concentration (2 to 12 g/l), flow rate (43.2 to 172.8 m/d), duration, and eventual alternation of injection and flushing periods was analyzed. The impact of each scenario was quantified in terms of particle mobility, porous medium clogging, water pressure, and uniformity of distribution of the particles in the porous medium. The results of this study indicate that, when injecting under conditions typical of a full-scale aquifer remediation, nanoparticle mobility and distribution are optimized and clogging is minimized by using high flow rates, low concentrations, and frequent injection steps without intermediate flushing.

Subject Headings

Colloids, Transport phenomena, Nanotechnology, Porous media, Groundwater, Remediation

Introduction

The injection of nanosized zerovalent iron particles (NZVI) represents one of the most innovative technologies for in-situ remediation of contaminated aquifer systems, thanks to the high reactivity of such particles towards a broad range of contaminants (Fiore and Zanetti 2009; Freyria et al. 2011; Phenrat et al. 2011; Saleh et al. 2008; Tosco et al. 2014; Zanetti and Fiore 2005; Zhang 2003; Zolla et al. 2009). However, the NZVI mobility in the subsurface is very limited when particles are injected without any surface amendment (eg adsorbed polymers, oil-in-water emulsions, particle coating with doping metals, etc.) (Johnson et al. 2009; Phenrat et al. 2010; Tiraferri et al. 2008; Tratnyek and Johnson 2006). Bare NZVI exhibits a strong tendency to aggregate, agglomerate, and consequently to settle rapidly onto the solid phase. This effect is especially marked for high NZVI concentrations (1 to 20 g/l), in the typical range required for field applications (Cantrell et al. 1997; Nurmi et al. 2005; Nyer and Vance 2001), and is extremely detrimental for field applications (Noubactep et al. 2012; Tiraferri and Sethi 2009). Excessive particle retention clogs the pores in the vicinity of the injection point, resulting in a significant loss of porosity and permeability, and in a very limited radius of influence of the injection. Continued injection under pressure will ultimately create preferential pathways, which strongly limit the contact between iron particles and contaminants, thereby reducing the effectiveness of remediation (Huang and Zhang 2005; Tosco and Sethi 2010; Westerhoff and James 2003). In recent years, great efforts have been consequently devoted to identify effective, non-harmful, and economically affordable strategies to improve the colloidal stability of NZVI dispersions (Tosco et al. 2014). The approaches explored so far include (i) modification of the particle surface properties to increase repulsion via adsorption of natural or synthetic polymers (starch, poly-acrylic acid, carboxymethyl cellulose, guar gum, etc.) (He and Zhao 2005; Kanel et

al. 2008; Phenrat et al. 2007; Schrick et al. 2004; Tiraferri et al. 2008); (ii) partial coating of the particle surface during synthesis (Sakulchaicharoen et al. 2010) or inclusion in oil emulsions (Quinn et al. 2005); (iii) improvement of the properties of the dispersant fluid using shear-thinning polymeric solutions (Comba et al. 2011; Comba and Sethi 2009; Dalla Vecchia et al. 2009; Johnson et al. 2009; Tiraferri et al. 2008; Xue and Sethi 2012), which reduce aggregation and sedimentation by increasing its viscosity.

Beside colloidal stability, a number of factors have also been identified that may significantly affect the mobility of NZVI in the subsurface, as summarized in Figure 1. These factors include the geochemical properties of the subsurface water and/or dispersing water (namely pH, ionic strength and dissolved species) (Bunn et al. 2002; Ryan et al. 2000; Saleh et al. 2008; Tiraferri et al. 2011; Tosco et al. 2009), physical characteristics of the aquifer (namely grain size, porosity and permeability) (Kim et al. 2007), spatial heterogeneities in the aquifer properties and contaminant concentration in the plume (Phenrat et al. 2011), depth of the contaminated area (Cook 2009), and specific characteristics of the NZVI, such as magnetic attractive forces (related to Fe^0 content) and size distribution (Phenrat et al. 2009). Finally, also the injection strategy, i.e. the operating conditions during NZVI injection, is known to control the mobility and the final fate of the injected particles. In particular, the injection rate (and thus the pore water velocity in dynamic conditions) (Bai and Tien 1999; Hosseini et al. 2011) and the delivered NZVI concentration are relevant parameters (Hosseini and Tosco 2013; Phenrat et al. 2010; Phenrat et al. 2009; Saleh et al. 2008).

The wide variety of factors reported in Figure 1 and the complexity of the possible interactions among them make a comprehensive study unfeasible (Chen et al. 2001). It should be also mentioned that, among the four sets of factors reported in Figure 1, only the characteristics of

particles and dispersant fluid as well as the injection strategy are fully in our control. Numerous researchers investigated the effects of one or more factors on the transport and retention of NZVI in well-controlled lab-scale columns (Nyer and Vance 2001; Phenrat et al. 2010; Saleh et al. 2008; Tiraferri and Sethi 2009), including particle stabilization methods, groundwater ionic strength and ion valence (Phenrat et al. 2010; Saleh et al. 2008; Tosco et al. 2012), particle size and composition, flow rate, and heterogeneities in the porous material (Hosseini and Tosco 2013; Kim et al. 2012; Phenrat et al. 2010). Conversely, few studies have been devoted to understand the role of the injection strategy, and to our knowledge no systematic study has been reported to date concerning the impact of different injection strategies (flow rate, NZVI concentration, injection duration and alternation with flushing) on NZVI mobility. When injecting NZVI slurries into the subsoil, the optimal injection strategy should provide a reasonable radius of influence (whose extent should be tunable by the operator) with the minimum impact of NZVI deposits on the porous medium permeability, to avoid the bypass of the reactive zone when natural flow conditions are restored. Phenrat *et al.* (2011) showed that, in the presence of a NAPL phase, NZVI cannot target the NAPL/water interface more efficiently when injected at lower flow rates and for a longer time, compared to fast single-step NZVI delivery.

In this study, a quantitative analysis is conducted on how the management of the injection of NZVI water-based slurries can optimize the mobility of the particles. In particular, the impact of injected NZVI concentration, flow rate, and number, duration, and alternation of injection and flushing periods is considered. NZVI transport simulations in 1D domains were performed using E-MNM1D (Tosco and Sethi 2010) for bimetallic nano-*Fe/Cu* particles, whose transport was previously assessed by the authors in laboratory column tests (Hosseini and Tosco 2013). Several injection scenarios were considered, including single-step injections (injection followed by

flushing), and multi-steps injections (repetition of injection+flushing steps) with constant and variable particle concentration. The performance of each scenario was quantified in terms of travel distance, changes in porous medium porosity, permeability, and overpressure during injection.

Material and Methods

The mobility and retention of highly concentrated dispersions of bimetallic *Fe/Cu* nanoparticles ($d_{50} = 70 \pm 5 \text{ nm}$, BET surface area $SA = 28.6 \text{ m}^2 \text{ g}^{-1}$, and particle density $\rho_p = 7550 \text{ kg m}^{-3}$) through columns (0.5 m length and 0.025 m inner diameter) packed with coarse sand ($d_{50} = 0.83 \times 10^{-3} \text{ m}$) was investigated in a previous work (Hosseini and Tosco 2013). In particular, the influence of high flow rate ($V = 43.2, 86.4, \text{ and } 172.8 \text{ m/d}$) and injected particle concentrations ($C_{inj} = 2, 5, 8 \text{ and } 12 \text{ g l}^{-1}$) was addressed. The ionic strength of flushing water (40 mM, constant during the experiments) mimicked the fresh groundwater used for the water supply of the city of Karaj (Iran). The experimental data (breakthrough curves and pressure drop logs over time) were modeled using the software E-MNM1D (Tosco et al. 2014; Tosco and Sethi 2010), which provides a numerical solution to colloid transport equations (advection-dispersion-deposition processes) coupled to clogging. E-MNM1D is available for free download at www.polito.it/groundwater/software both as a Matlab-based code with Excel interface, and as a part of the colloid transport simulation software MNMs 2014 (with a graphical interface).

Colloid transport equations and numerical solution

E-MNM1D (Tosco and Sethi 2010) solves the 1-D colloid transport equation in saturated porous media with two concurrent kinetics of particle deposition and resuspension to and from the soil

matrix. The set of partial differential equations describing the coupled flow and transport of colloidal suspensions with associated clogging can be written as follows:

$$\frac{\partial}{\partial t}(\varepsilon C) + \frac{\partial}{\partial t}(\rho_b s) = \frac{\partial}{\partial x}(qC) + \frac{\partial}{\partial x}\left(\varepsilon D_x \frac{\partial C}{\partial x}\right) \quad (1)$$

$$\frac{\partial}{\partial t}(\rho_b s_1) = \varepsilon k_{a1} (1 + A_1 s_1^{\beta_1}) C - \rho_b k_{d1} s_1 \quad (2)$$

$$\frac{\partial}{\partial t}(\rho_b s_2) = \varepsilon k_{a2} \left(1 + \frac{x}{d_{50}}\right)^{\beta_2} C - \rho_b k_{d2} s_2 \quad (3)$$

$$\varepsilon(s) = n - \frac{\rho_b}{\lambda \rho_p} s \quad (4)$$

$$a(s) = a_0 + \theta a_p \frac{\rho_b}{\rho_p} s \quad (5)$$

$$K(s) = K_0 \left[\frac{\varepsilon}{n}\right]^3 \left[\frac{a_0}{a}\right]^2 \quad (6)$$

$$-\nabla p = \frac{\mu}{K(s)} q \quad (7)$$

where x [L] and t [T] are the independent variables for space and time, respectively; C [ML^{-3}] and s [M/M] are the concentration of particles suspended in the fluid phase and deposited on the soil matrix, D_x [L^2T^{-1}] is the hydrodynamic dispersion, q [LT^{-1}] the Darcyan flow velocity, ρ_b [ML^{-3}] the porous medium bulk density, ε [-] the effective porosity, k_a^i and k_d^i [T^{-1}] the deposition and release rate coefficients for the i^{th} interaction site ($i=1, 2$), A_i and β_i [-] the multiplier and exponent coefficients defining the interaction dynamics, d_{50} [L] the mean diameter of the porous material, n [-] the initial porosity of the porous medium before the injection of the particles, a_0 [L^{-1}] the

specific surface area of the porous matrix, $a_p [L^{-1}]$ the specific surface area of the nanoparticles, $\rho_p [ML^{-3}]$ the density of the nanoparticles, $\lambda [-]$ the average degree of packing of the particle deposits, θ the fraction of deposited nanoparticles contributing to the overall increase of the interface area [-], $K(s)$ the permeability coefficient expressed as a function of the concentration of deposited particles [L^2], K_0 the permeability coefficient in the absence of deposited particles, μ the dynamic viscosity of the pore fluid [$ML^{-1}T^{-1}$], q the darcyan flow velocity [$L T^{-1}$], and ∇p the pore pressure gradient [$ML^{-1}T^{-2}$].

Two reversible particles deposition/release sites were considered ($S = S_1 + S_2$): site 1 is a generic formulation which can be adapted to all commonly used interaction kinetics (first-order deposition dynamics if $A_1 = 0$, blocking if $A_1 < 0$, ripening if $A_1 > 0$) and site 2 considers the space-dependent deposition dynamics following straining kinetics (Bradford et al. 2003). The reader can refer to previous works for a detailed discussion on the colloid transport mechanisms and model equations (Tosco et al. 2014; Tosco and Sethi 2009; Tosco and Sethi 2010).

The following initial conditions for $C(x,t)$ and $s_i(x,t)$ were applied:

$$C(x,t=0) = 0 \quad (8)$$

$$s_i(x,t=0) = 0 \quad i = 1, 2 \quad (9)$$

A zero-gradient boundary condition was applied at the column outlet ($x = L$). To define the boundary condition at the column entrance ($x=0$), different particle-injection strategies were employed. Each strategy includes alternated stress periods of particle injection (with particle concentration C_{inj} constant during the stress period) and flushing (without particle injection). The model provides results in terms of particle transport (breakthrough curves and profiles of

deposited iron concentration), water pressure drop over time, changes in the effective porosity, and permeability over space and time.

Transients in the flow field were assumed negligible, and consequently the problem was solved as a quasi-stationary phenomenon, and Darcy's law was applied to calculate the pressure gradient. In the numerical solution of the set of model equations, the flow rate was updated for each stress period and assumed equal to the imposed inlet flow rate, while permeability was calculated at each time step.

Injection strategies of nano-Fe/Cu particles

In a previous work (Hosseini and Tosco 2013), the inverse modeling of experimental results with the equations (1-9) provided three sets of transport parameters, one for each flow rate, which were found independent of the injected concentration. In this study, such values of the transport parameters were used in the transport simulations (Table 1). The three flow rate values were selected as representative, respectively, of near-well conditions, intermediate distance, and longer distance from the injection well (Table 2), following Johnson *et al.* (2009). Please note that the parameter A_1 is positive for all applied flow rates, suggesting that the deposition of particles onto the porous medium follows a ripening dynamics.

Twenty five different injection strategies were defined (Table 2, 3 and 4). For each strategy, a transport simulation was run using E-MNM1D in direct (predictive) mode, and the results were analyzed in terms of eluted and retained NZVI concentration, variation in pressure drop, effective porosity, and porous medium permeability over time. For each strategy, three simulations were run imposing three pore water velocities (43.2, 86.4, and 172.8 m/d) for a total number of 75 (25×3) simulations. The flow rate was kept constant during each simulation.

To make the results of different injection strategies comparable among each other, the overall duration of the simulated experiment, t_{exp} (i.e. duration of injection + flushing steps), and the total injected mass of NZVI, TIM , were the same in all simulations (see also Tables 3 and 4):

$$t_{exp} = \sum_{i=1}^n t_{inj}^i + \sum_{j=1}^m t_{flush}^j = 40000 \text{ s} \quad (10)$$

$$TIM = \sum_{i=1}^n F_{inj}^i \times t_{inj}^i = 32600 \text{ kg/m}^3 \quad (11)$$

where n and m are respectively, the number of injection and flushing periods in each strategy. It should be mentioned that the value of $t_{exp}=40000$ s is valid only for pore water velocity of 43.2 m/d, whereas for the cases of 86.4 and 172.8 m/d (t_{inj}) must be reduced to impose the same injection fluxes.

The experimental results reported in the previous study (Hosseini and Tosco 2013) evidenced that, for high NZVI concentrations (particularly 8 and 12 g/l), the pressure drop at column ends, which is directly related to clogging phenomena, is expected to significantly increase after ~600 seconds. Therefore, pore plugging can be minimized if NZVI injection is stopped before the pressure begins to grow. For this reason, injection times lower than 600 seconds were considered in the strategies herein investigated. However, for two strategies (strategies #S2 and #S3 as define following) a prolonged injection time was adopted for comparison.

More in details, the injection strategies can be classified into four sets based on several prior trails:

1. First set: strategies #S1 to #S3 (Table 3): experiments include two stress periods (Figure 2a), namely one injection followed by one flushing step, characterized by different duration, NZVI mass flux F_{inj} , and injected particle concentration C_{inj} .

2. Second set: strategies #S4 to #S11 (Table 3): the effects of one flushing step between two injections and of different injected concentrations are investigated (Figure 2b-d). The overall duration of the injection is the same for all tests, while the duration of flushing periods varies.
3. Third set: Strategies #S12 to #S22 (Table 3): experiments include three injection steps, each followed by a flushing step. (Figure 2e-h). Also in these cases, the overall duration of the three injection stress periods is the same for all tests, regardless the injected concentration, while the injected concentration and the duration of flushing steps varies.
4. Fourth set: Strategies #S23 to #S25 (Table 4): the effects of a gradual variation of injected concentration is investigated, simulating the continuous injection of particles divided into nine injection steps characterized by a gradual variation in the NZVI concentration (Figure 3).

Evaluation Criteria

For comparison among the simulation results of the 25 strategies, the following evaluation criteria were calculated:

- Criterion $C1_{\Delta P}$: Percentage of increase in maximum pressure drop: for a given strategy; this criterion compares the peak in pressure drop at column ends (due to clogging) with the initial (clean bed) pressure drop (ΔP_0):

$$C1_{\Delta P} = \frac{\Delta P_{max} - \Delta P_0}{\Delta P_0} \times 100 \quad (12)$$

where ΔP_{max} is the maximum pressure drop reached during the simulation.

The pressure drop along the column is directly related to the pore water velocity through the Darcy's law. In clean bed conditions (i.e. no particles retained), a pressure drop equal to $\Delta P_0 = 1114.08, 2228.16, \text{ and } 4456.34 \text{ Pa}$ was calculated, for the three considered flow velocities $V=43.2, 86.4, \text{ and } 172.8 \text{ m/d}$, respectively.

- Criterion $C2_{C/C_{inj}}$: Percentage of retained mass: the percentage of total mass of NZVI retained within the 0.5 m long domain at the end of the simulation is considered. The integral of the normalized breakthrough curve C/C_{inj} was used to calculate the mass balance:

$$C2_{C/C_{inj}} = \left(1 - \frac{\int_0^{t_{exp}} c/c_0 dt}{\sum_{j=1}^n t_{inj}^j} \right) \times 100 \quad (13)$$

where n is number of injection periods for the given strategy.

- Criterion $C3_{AEP}$: Percentage of reduction in averaged effective porosity. The effective porosity averaged along the column (AEP), directly related to the pressure drop, was calculated as the integral of the space-dependent porosity:

$$AEP(t) = \frac{1}{L} \int_0^L \varepsilon(x, t) dx \quad \text{for } t=0 \text{ to } t_{exp} \quad (14)$$

where L is the column length. In the $C3_{AEP}$ criterion, the maximum loss in AEP was calculated as the ratio of the minimum AEP (AEP_{min}) to the initial AEP ($AEP_0 = n=0.37$):

$$C3_{AEP} = \frac{AEP_0 - AEP_{min}}{AEP_0} \times 100 \quad (15)$$

- Criterion $C4_K$: Percentage of reduction in permeability, which is related to the normalized effective porosity and normalized specific surface area of the porous medium through the Kozeny-Carman equation (eq. 6). The average permeability along the column at the end of the experiment (K_a) was calculated by applying an integral in space (similarly to eq. 14) and comparing the result to the initial (clean bed) permeability ($K_0 = 5.6 \times 10^{-11} m^2$):

$$C4_K = \frac{K_0 - K_a}{K_0} \times 100 \quad (16)$$

All the criteria refer to different critical effects of NZVI injection, and can be directly applied to compare the different strategies: the lower the values, the better the performance of the injection strategy in terms of improving mobility of the injected NZVI and minimizing clogging. As a further criterion, the distance over which 99% of the particles are retained ($L_{99\%}$) was calculated ($C5_{L99\%}$). To this aim, the model domain was extended up to several meters in order to get a zero breakthrough at the exit in all conditions, and the travel distance was calculated from the simulated profiles of retained particle concentration. For a better comparison of the efficacy of the considered strategies based on the criterion $C5_{L99\%}$, the injection strategy which produced the lowest $L_{99\%}$ was considered as the benchmark (referred to as l_b) and the travel lengths obtained from all other strategies were evaluated against this benchmark, and expressed as a relative travel distance with respect to l_b .

Results and Discussion

Criterion $C_{1\Delta P}$: pressure drop

The five criteria were recalculated from the corresponding simulated results of EMNM1D for each strategy and flow rate. The values of criterion $C_{1\Delta P}$ (percentage of increase in maximum pressure drop) are summarized in the bar-plot of Figure 4. Consistently with Darcy's law, the maximum pore water pressure increases with increasing pore water velocity. This effect is more relevant for strategies S1, S2 and also S6 to S11. The fourth set of strategies S23-S25 (S25 in particular) corresponds to the minimum increase in pressure drop (i.e. minimum $C_{1\Delta P}$), while the strategy S1 corresponds to the maximum drop. For comparison, Figure 5 reports the profile of water pressure drop versus time of the simulated injections for strategies S1 and S23 for $V=43.2$ and $V=172.8$ m/d. In S1, a constant NZVI concentration equal to 16 g/l was injected for 545 s, while in S23 an average concentration C_{inj} of 1.8 g/l, with peak concentration of 3.3 g/l, was injected for an overall duration of $525 \text{ s} \times 9 = 4725 \text{ s}$. In other words, the injection of lower concentrations with a gradual variation of C_{inj} (in a triangular shape) has the lowest hydrological impact compared to particles injected at higher and constant concentration. Coherently, among strategies S1 to S3 the highest pressure increase is registered for S1, which is associated to the highest injected concentration, and decreases with increasing C_{inj} . This observation is consistent with the simulated ripening kinetics for particle attachment, and with experimental results of laboratory injection tests, which indicated an increase in porous medium clogging with increasing concentration of injected suspension (Hosseini and Tosco 2013). However, the simulation results also indicate that, as a general rule, the overall increase in pressure drop is limited when the injection of high concentrations is associated to a short duration, and injection steps are spaced out by flushing steps (strategies S4 to S22): this result is suggested by the lower values of

$C_{1\Delta P}$ criterion for the strategies S12-S21 (three injection steps, maximum injected concentration 12.0 g/l) compared to strategies S6 to S11 (two injection steps, maximum injected concentration 12.8 g/l). Conversely, a higher injection rate heightens the pressure drop during NZVI injection (Figure 5) with negative hydrological effects, but conversely it shortens the flushing time required to remove clogging and to recover the initial conditions.

Criterion $C_{2C/C_{inj}}$: retained particles

Figure 6 shows the results of criterion $C_{2C/C_{inj}}$, which represents the percentage of injected particles retained within 0.5 m from column inlet. The results indicated that strategies S3, S23, S24, and S25 will result in higher mobility of the particles, with lower retained mass. This result also suggests that a lower injection rate increases the mobility of the particles. In particular, strategies S23, S24, and S25 correspond to the lowest values of $C_{2C/C_{inj}}$ (retained mass lower than 3.7%) for the highest pore water velocity ($V = 172.8$ m/d), whereas the strategy S1 at the lowest flow rate corresponds to the highest value of $C_{2C/C_{inj}}$ (93.5%) and therefore to the worst performance. As a general rule, for a given injection strategy, increasing the flow velocity has a significant effect on the mobility of particles, but also the injection strategy has a major impact (see Figure 6).

Examples of breakthrough curves (BTCs) for different injection strategies and flow rates are reported in Figure 7. A multi-modal behavior was observed in the BTC curve during flushing for some strategies, particularly at high flow rates. This behavior corresponds to the concurrence of two different release mechanisms, which were experimentally observed for the nano-Fe/Cu particles. At the early stages of flushing, a first rapid release was registered, while a second, delayed peak was observed during advanced flushing. This result implies that two different

release mechanisms are occurring in such conditions, namely fast and slow detachment, respectively related to sites 1 and 2 in equations 2 and 3. This phenomenon becomes more relevant when increasing the pore water velocity (Figure 7 a).

Figure 8 reports the final profiles of retained particles at the end of the simulation for selected scenarios. It is evident that after the last stress period ($t=t_{exp}$), the particles are mainly retained in the second part of the domain, since those in the first part have already been (at least partly) flushed out. In addition, the strategies associated with the longest flushing (10000 s) between two subsequent injection periods (strategies S16, S13, S22, S11, S19, S11, S8, and S5) correspond to the most heterogeneous particle distribution along the column (Figure 8a), due to the alternation of deposition and release. Also, enhancing the flow velocity (from 43.2 to 86.4 m/d) results in an enhanced NZVI breakthrough at the column outlet (see also Figure 7), and consequently fewer retained particles (Figure 8 b) are observed. It is finally worth to mention that, when injecting at higher flow rates, the differences between particle profiles obtained from different strategies are enhanced.

Criterion $C3_{AEP}$: porosity reduction

Figure 9 shows the results of the criterion $C3_{AEP}$ (percentage of reduction in averaged effective porosity AEP) for all strategies with different pore water velocities. The fourth set of strategies (S23 to S25) corresponds to the minimum AEP reduction (0.22% for S23 when $V=43.2$ m/d), while the first strategy (S1) has the maximum value of $C3_{AEP}$ (1.70% when $V=172.8$ m/d). Lower injection rates and flushing periods between the injections limit the decrease in AEP. The variation of the AEP as a function of time for strategies S1 (for $V=43.2$ and 172.8 m/d) and S25 (for $V=43.2$ m/d) is reported in Figure 10.

Criterion C_{4K}: permeability reduction

The criterion C_{4K} describes the variation of the average permeability (K) along the domain at the end of experimental time and is strictly related to criterion C_{3AEP}, since changes in porosity are, along with variations in the solid-liquid interface area, one of the two components contributing to changes in permeability. The obtained results are given in the bar-plot of Figure 11 in semi-logarithmic scale. The maximum and minimum values of C_{4K} for the three flow velocities are associated, respectively, to strategy S13 (for $V=43.2$ m/d) and S25 (for $V=172.8$ m/d). This finding suggests again that clogging is more pronounced when higher inlet concentrations are applied, even if the total injected mass is the same. In addition, decreasing the pore water velocity has a considerable effect on the reduction of sand permeability at the end of the experiment. Figure 12 reports examples of the final profiles of K/K_0 as a function of time for selected injection strategies.

Criterion C_{5L99%}: travel distance

The final criterion C_{5L99%} is associated to the distance over which 99% of the particles are retained through the column. The obtained results are presented in Figure 13 for different flow velocities. The lowest particle mobility is associated with strategy S1 for a flow velocity of 43.2 m/d, and is equal to approximately 2.0m. Assuming this value as the benchmark (l_b), the travel lengths $L_{99\%}$ obtained for the other strategies are reported as normalized travel distances $L_{99\%}$ in Figure 13. For $V=43.2$ and 86.4 m/d, the strategy S3 is associated to the maximum travel length ($1.2 l_b$ and $2.3 l_b$), but the impact of the strategy on $L_{99\%}$ is limited compared to that of flow rate:

doubling the flow velocity from 43.2 to 86.4 m/d results in almost doubling of the travel length. Increasing the flow velocity to $V=172.8$ m/d, the impact of the injection strategy is more evident. The strategy S23 corresponds to the maximum particle mobility, equal to $L_{99\%}=5.6l_b$. In general, the injection strategies S23-S25, associated to longer injections at lower concentrations, evidence a longer travel length compared to the other scenarios. This observation suggests that the combined effect of high flow rate and low concentration, which corresponds to a lower deposition rates, promotes the mobility of the particles.

Finally, Figure 14 provides a comparison of the effects of flow rate, injected concentration, and number of injection steps on $L_{99\%}$. Figure 14a shows that, the travel length increases significantly with decreasing concentration of injected suspensions (e.g. S1, S2, S3), as discussed above. Conversely, intermediate flushing steps have a minor impact on $L_{99\%}$: as an example, the reader can compare the travel length for S2-S4 (with one intermediate flushing) and S12 (with two intermediate flushing). Also, Figure 14b indicates that multi-step injection with increasing C_{inj} (S14 and S17) results in shorter travel distances if compared to multi-step injection with decreasing C_{inj} (S23 to S25).

In conclusion, it is worth to mention that the results of criterion $C5_{L99\%}$ for some scenarios may overestimate the travel length of the nanoparticles when up-scaled to natural aquifer conditions, since in a field injection the flow rate is not constant with changing distance from the injection point, according to the radial flow pattern and aquifer heterogeneity. However, injection strategies resulting in longer travel distances in 1D simulations are also likely to be related to longer travel distances in the field, and this criterion applied to simple 1D simulations can provide a first qualitative indication of the expected radius of influence of a NZVI injection in the field.

Remarks and Conclusions

The transport simulations of nano-*Fe/Cu* particles in 1D porous media provided indications on the influence of different injection strategies on the overall mobility of the particles and clogging of the porous medium. In particular, the effects of alternating injection and flushing steps, of constant or variable inject concentration, and of flow rate and injection duration evidenced that all these phenomena are relevant when identifying the best strategy for NZVI delivery. As a general rule, the injection of larger volumes of NZVI dispersion at lower particle concentration has the minimum impact on the hydrological properties of the porous medium, and results in a more pronounced mobility of the particles. In addition, the pore plugging can be minimized if the injection is limited in time, and if it is stopped before the pressure drop begins to dramatically increase. Under such conditions, further prolonging the injection does not result in a significant increase of the NZVI travel distance, but only in a reduction of porosity and permeability, as evidenced also in simulations of microscale iron injection in radial domains (Tosco et al. 2014). On the other hand, the results of this study also indicate that a gradual variation of particle concentration without intermediate flushing can result in limited clogging and enhanced traveling distance.

Despite these conclusions, maximizing travel length and minimizing clogging are not the only aspects to be taken into account for a successful NZVI application at the field scale. As an example, the stoichiometric ratio of Fe^0 content to the contaminant (*Fe/C*) plays a crucial role in the contaminant reduction, and consequently in the remediation process. Therefore, a limit to the decrease of the injected NZVI concentration has to be fixed in order to guarantee a complete remediation. Moreover, it is also important to limit the total injected volume of NZVI slurry. The

injection of large volumes of slurry (and eventually of water, if intermediate flushing steps are applied) may result in long injection operations, high costs, and possible problems in handling large volumes of diluted slurries at the site. Furthermore, if the contaminant is present mainly as dissolved phase, injecting large volumes may result in displacing most of the contaminated water around the injection point. In this case, a further extraction step could be necessary few days after NZVI delivery, to “pull back” the contaminant in the NZVI reactive zone (Velimirovic et al. 2014). This process would increase the time, the cost, and the risks associated to the remediation. Therefore, a trade-off state exists between the aspects discussed above: the optimal injection strategy is likely not to be the one which can maximize mobility alone, nor minimize clogging, or the injected volume, but a reasonable compromise among a significant radius of influence with a fairly homogeneous distribution of NZVI, limited clogging, limited volume of iron to be injected, and a final NZVI concentration sufficient to guarantee fast degradation of the contaminants.

The results herein presented are not to be intended as universal from a quantitative point of view, since they are obtained from transport simulations of a specific type of NZVI. However, the conclusions drawn here by comparing the different injection strategies can be extended to other colloidal systems. Moreover, a general approach for the identification of effective injection strategies in the delivery of nanoscale particles can be proposed. This approach involves the performance of few targeted column tests, aimed at defining the model coefficients in the range of interest of colloid concentration and flow rate via inverse fitting, and then the application of the transport model in direct mode to simulate different injection scenarios. Further modeling for a more quantitative prediction of the expected radius of influence, particle distribution, and porous medium clogging can then be performed on few selected injection scenarios using more

complex colloid transport simulation tools. In this sense, for example, MNMs can be used, which implements the transport equations (1-7) in radial geometry (Tosco et al. 2014). The approach herein described can be adapted to any kind of colloidal dispersion, and in particular to concentrated suspensions that have strong interactions both between particles and porous medium and among particles themselves (straining, ripening, and consequent clogging), like most dispersions of nanoscale and microscale iron-based particles used in groundwater remediation.

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Figures

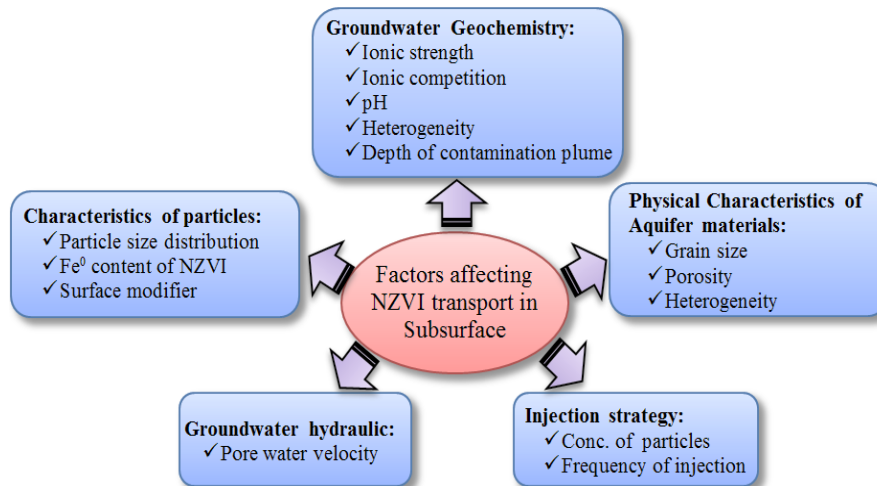


Figure 1: Factors affecting NZVI transport in saturated porous media

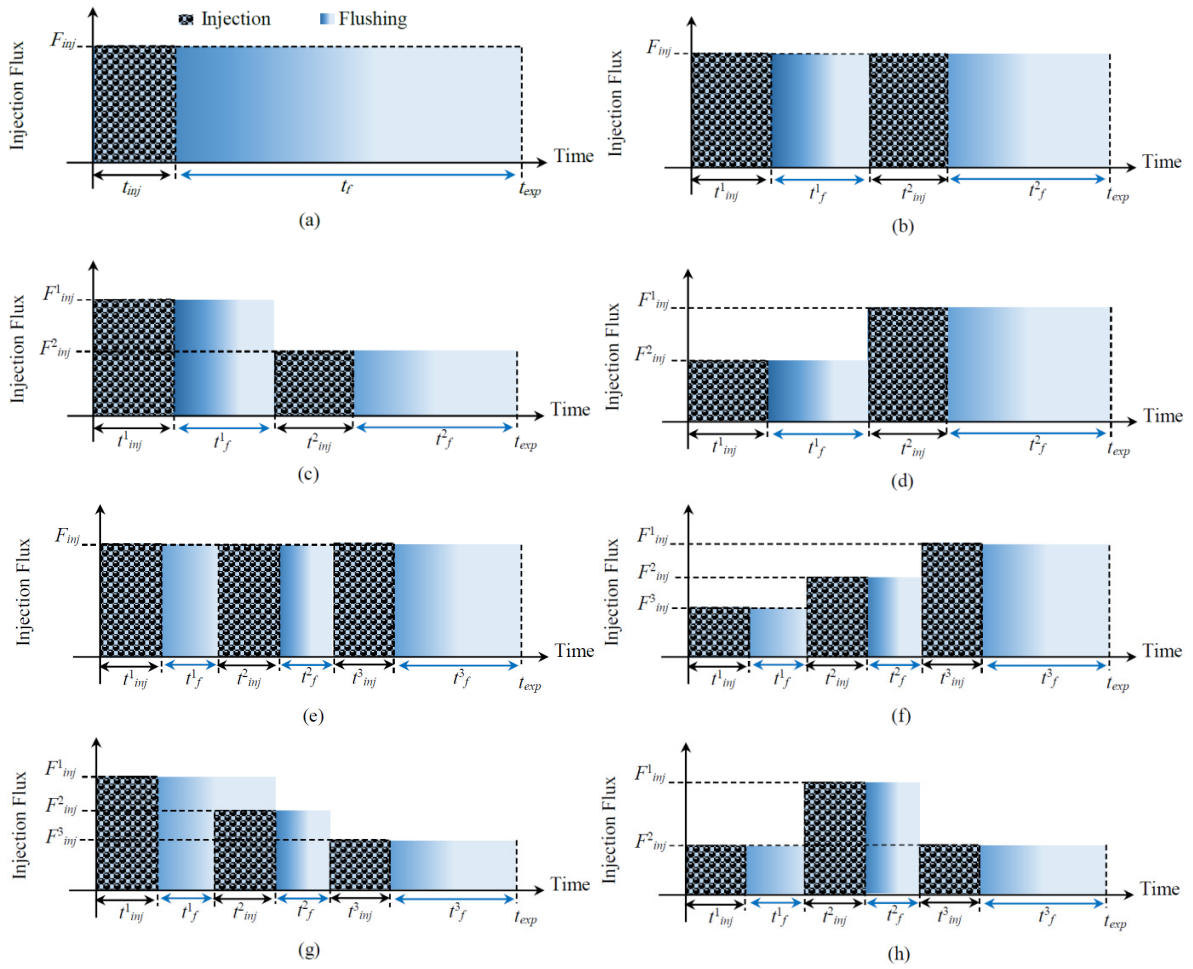


Figure 2: Configuration of the injection strategies for set 1 (a), set 2 (b-d), set 3 (e-h).

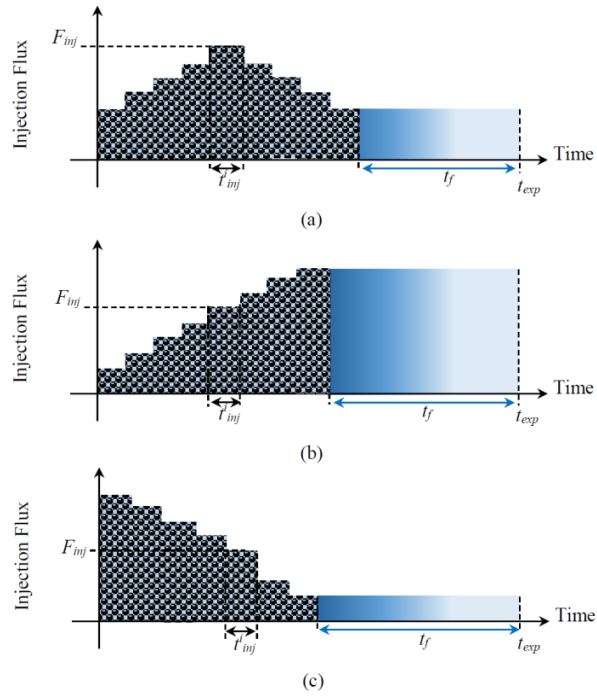


Figure 3: Configuration of the injection strategies of set 4.

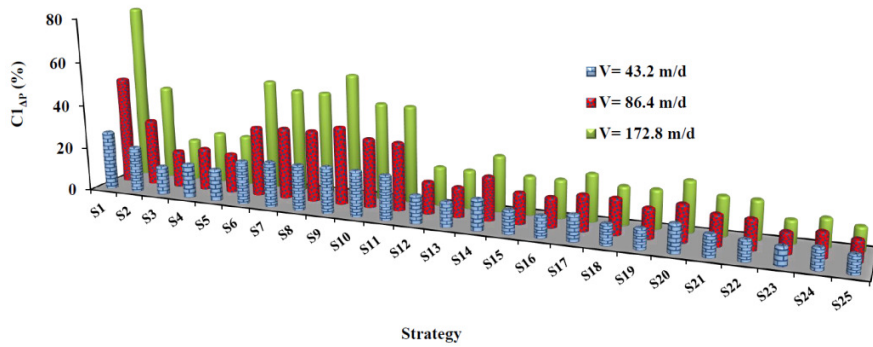


Figure 4: Results of the criterion $Cl_{\Delta P}$ for different strategies and pore water velocities.

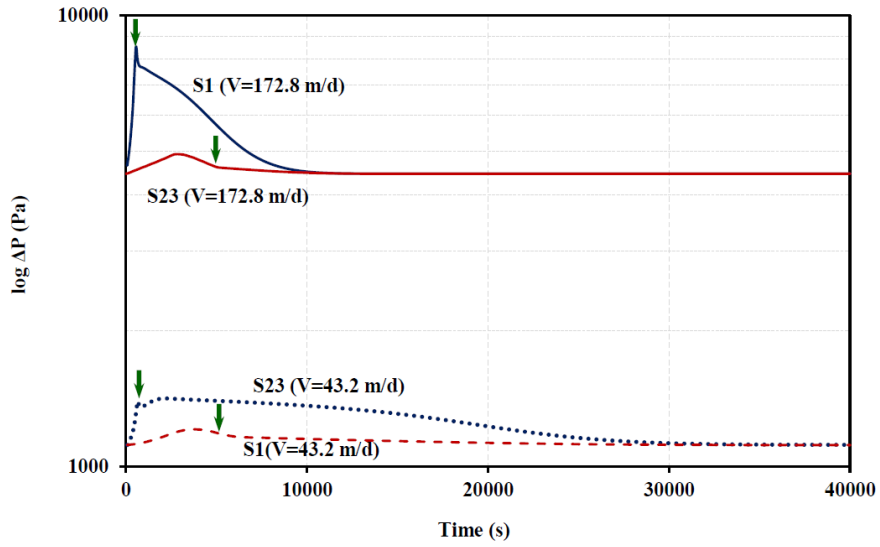


Figure 5: Pressure drop at column ends as a function of time for strategies S1 and S23 with $V=43.2$ and $V=172.8$ m/d. The arrows show the end of the NZVI injection in each simulation.

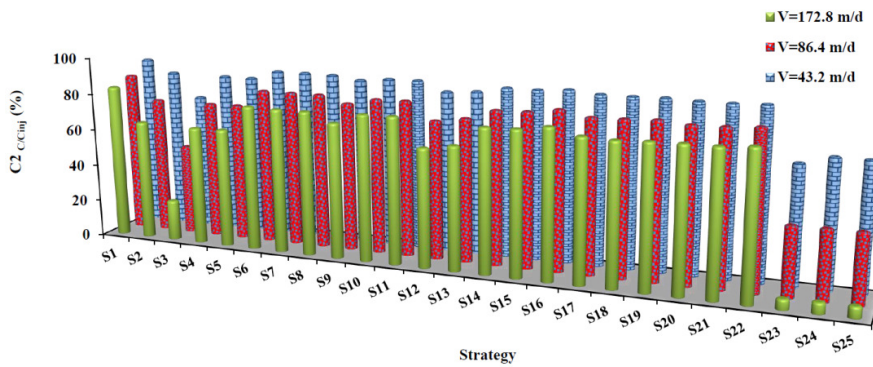
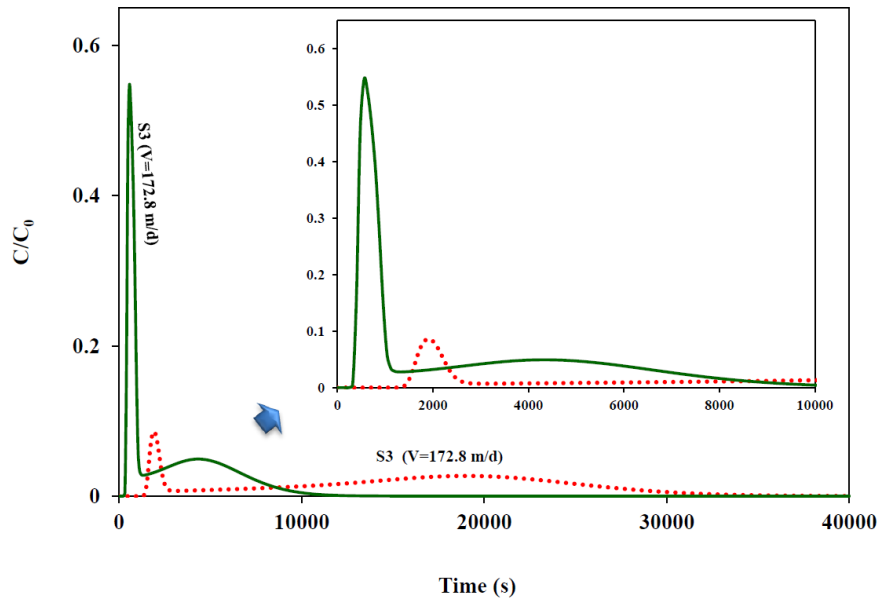
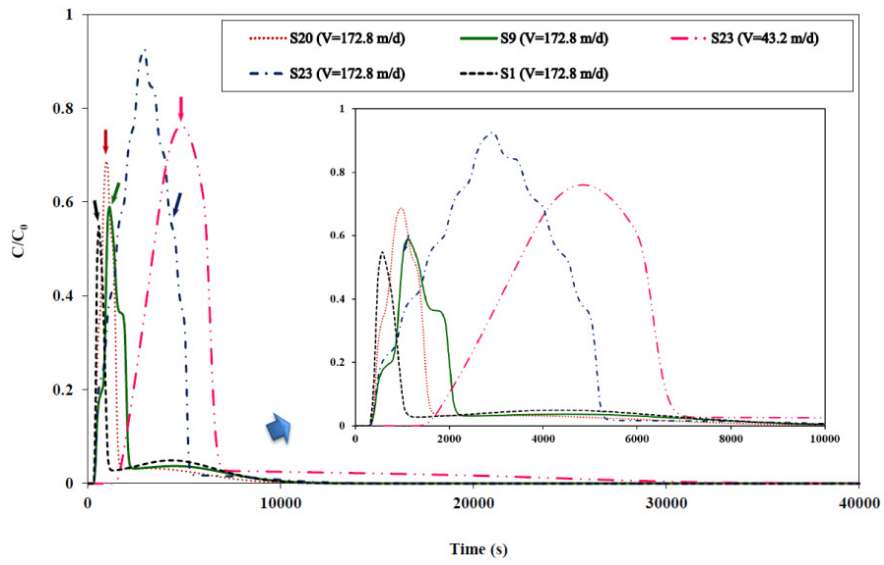


Figure 6: Results of the criterion $C2_{C/Cinj}$ for different strategies and pore water velocities.

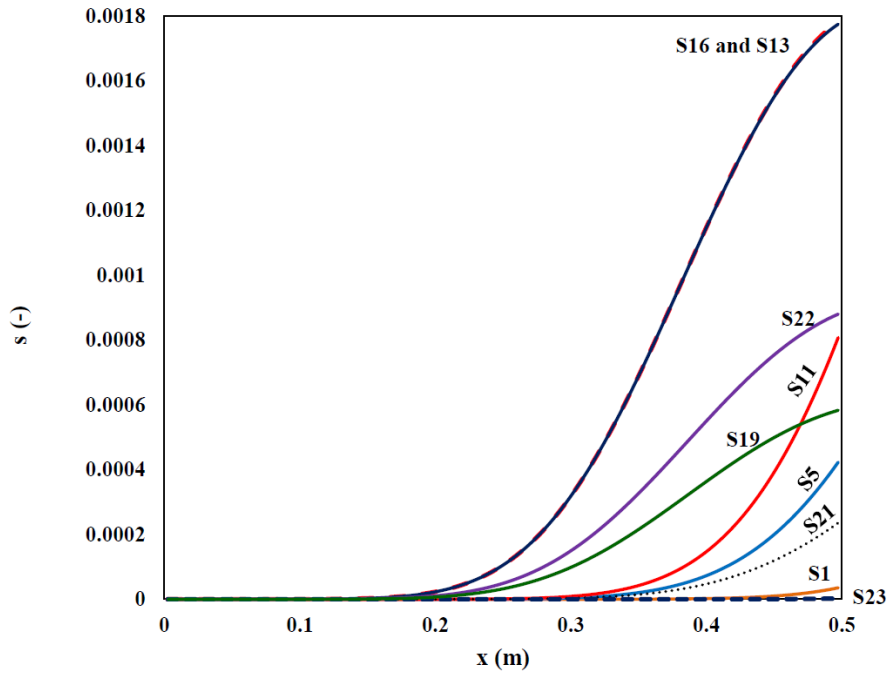


(a)

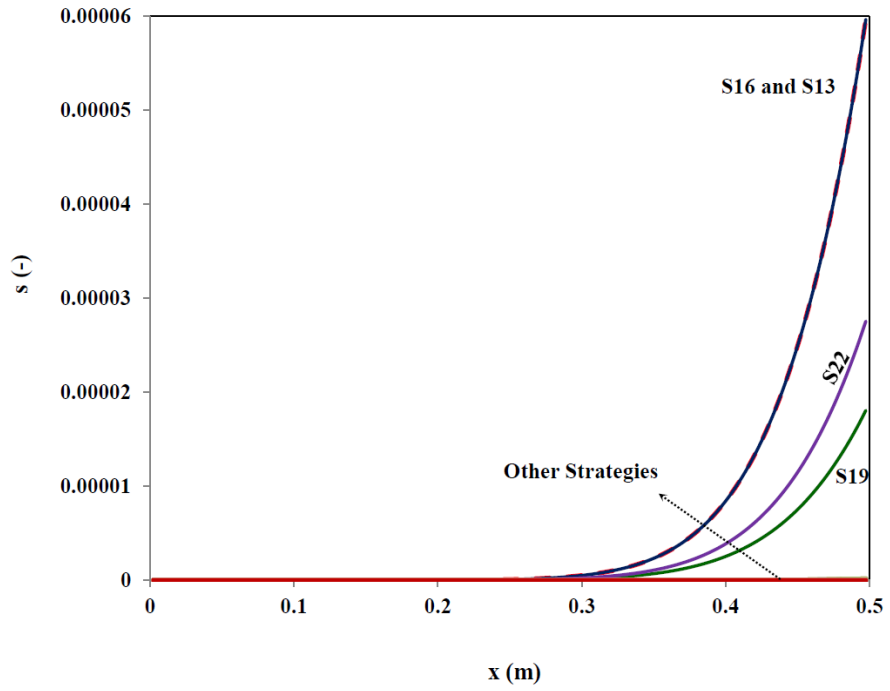


(b)

Figure 7: BTCs at 0.5 m for different strategies at different velocities.



(a)



(b)

Figure 8: Examples of final profiles of particle concentration for pore water velocities of a) $V=43.2$ m/d, and b) $V= 86.4$ m/d

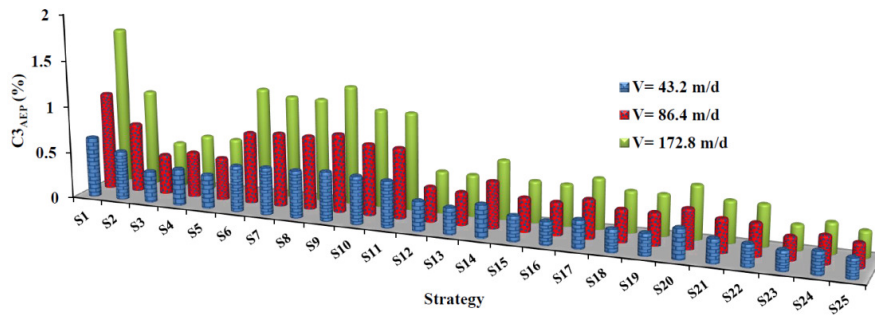


Figure 9: Results of the criterion $C3_{AEP}$ for different strategies and pore water velocities.

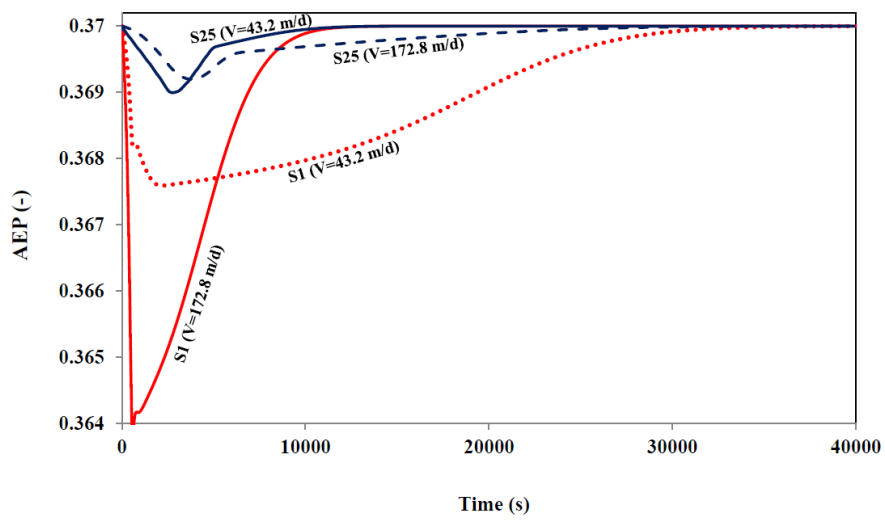


Figure 10: Variation of AEP as a function of time for strategies S1 ($V=43.2$ and 172.8 m/d) and S25 ($V=43.2$ and 172.8 m/d).

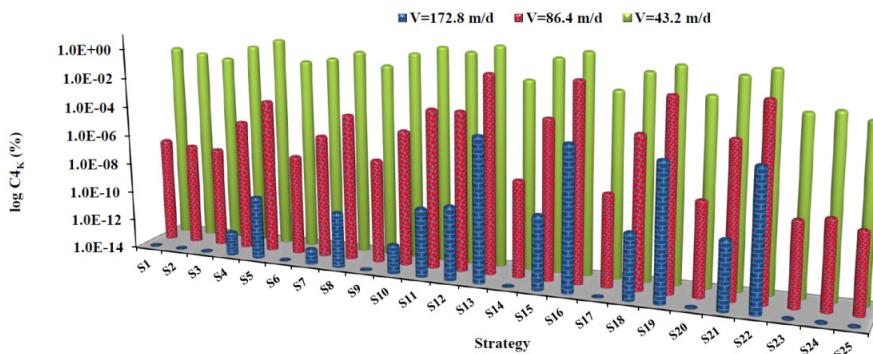


Figure 11: Results of the criterion $C4_K$ for different strategies and pore water velocities.

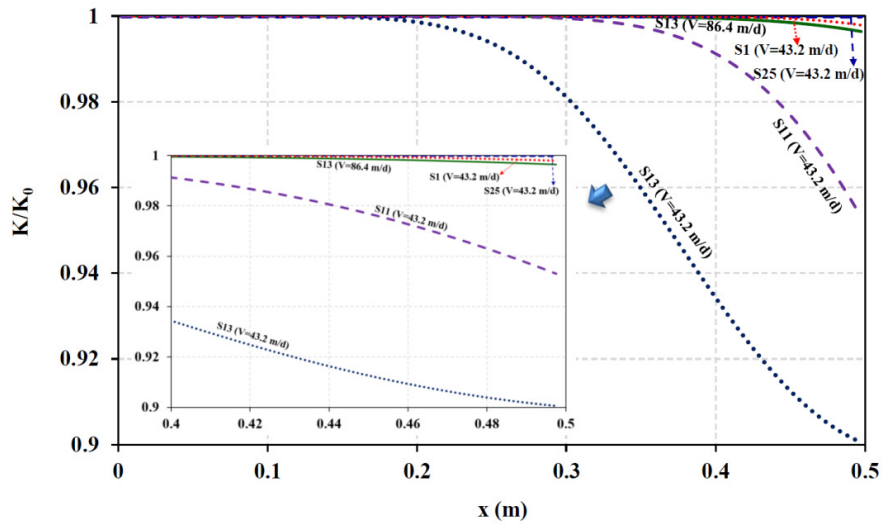


Figure 12: Examples of variation of K/K_0 through the column length for selected strategies at the end of the simulation (t_{exp}).

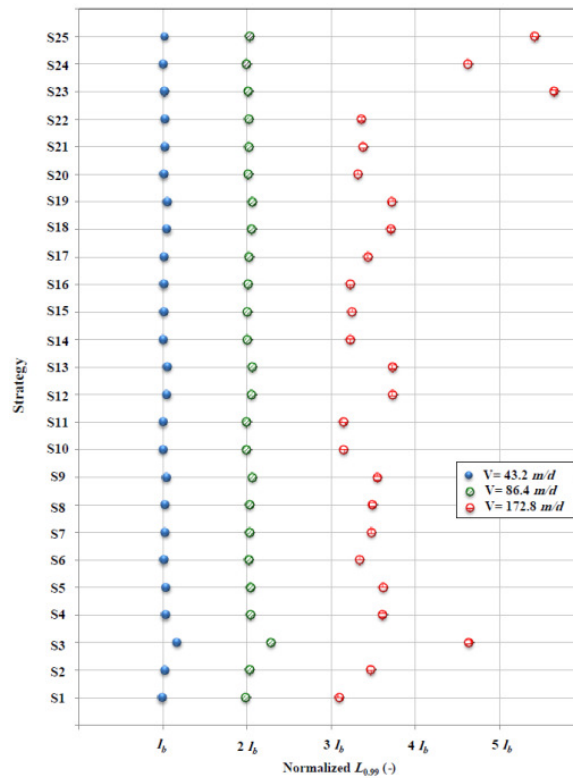
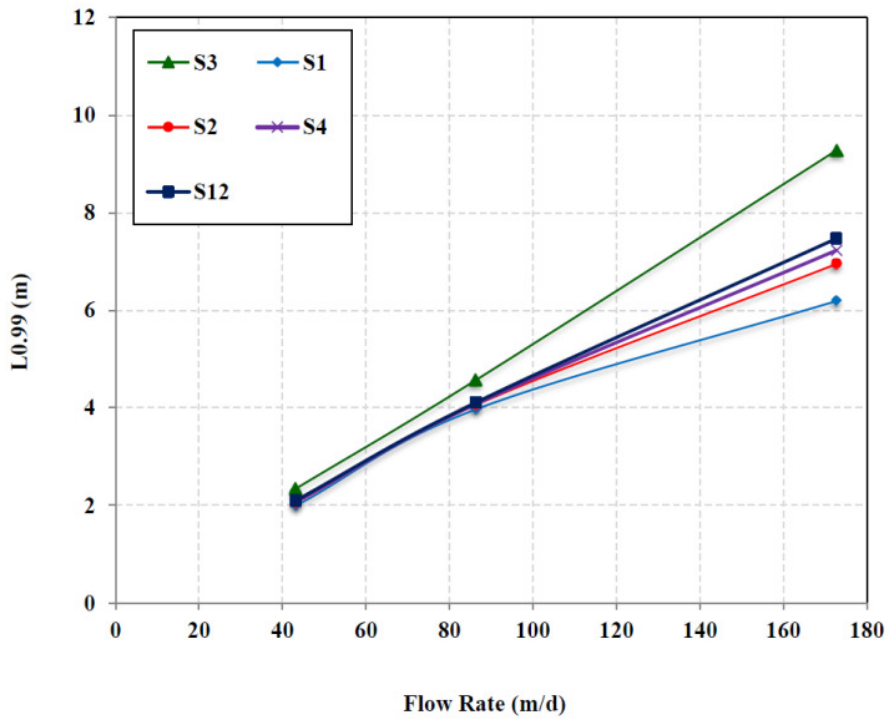
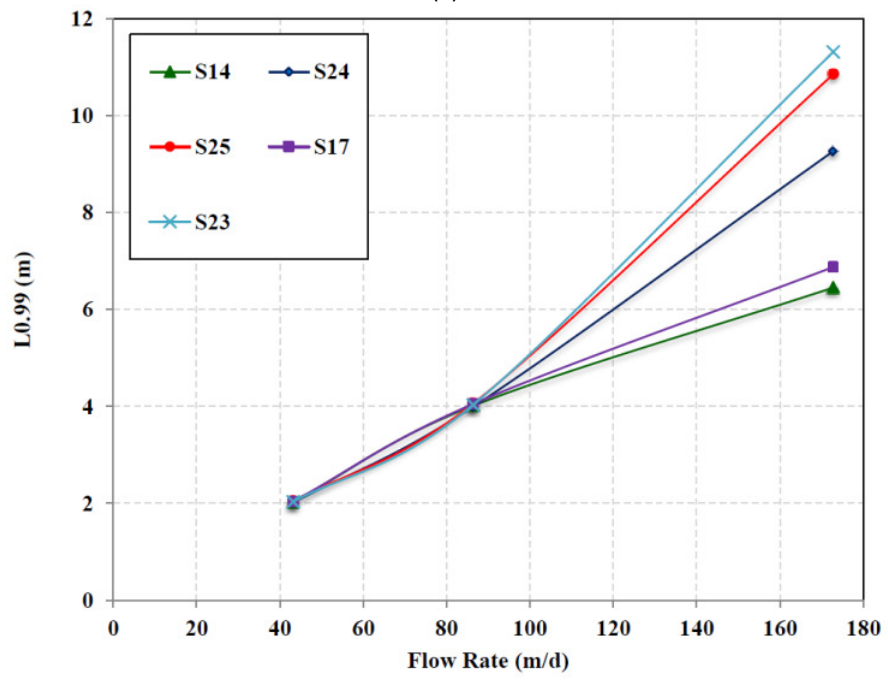


Figure 13: Normalized distance over which 99% of the particles are retained ($C5_{L99\%}$) referred to the benchmark value ($l_b=2\text{ m}$) for different strategies and flow rates.



(a)



(b)

Figure 14: Comparison of travel lengths $L_{99\%}$ for selected injection strategies.

Tables

Table 1: Transport parameters from experimental data fitting (data from Hosseini and Tosco 2013) used in transport simulations with E-MNM1D.

Parameter (unit)	Parameter Explanation	Value		
		V=43.2 m/d	V=86.4 m/d	V=172.8 m/d
λ (-)	Average degree of packing of the particle deposits	0.371	0.354	0.354
θ (-)	Fraction of deposited nanoparticles contributing to the overall increase of the interface area	0.0036	0.0038	0.0050
k_{a1} (s^{-1})	Deposition rate coefficients for the interaction site 1	0.0060	0.0128	0.0130
k_{d1} (s^{-1})	Release rate coefficients for the interaction site 1	0.016	0.032	0.039
k_{a2} (s^{-1})	Deposition rate coefficients for the interaction site 2	0.042	0.028	0.024
k_{d2} (s^{-1})	Release rate coefficients for the interaction site 2	0.0030	0.0020	0.0032
A_1 (-)	Multiplier coefficients defining the interaction dynamics for site 1	3300	3685	4000
β_1 (-)	Exponent coefficients defining the interaction dynamics for site 1	1.460	1.485	1.510
β_2 (-)	Exponent coefficients defining the interaction dynamics for site 2	0.001	0.012	0.170

Table 2: Typical flow velocity as a function of distance from a pumping well, data from Johnson *et al.* (2009)

Groundwater Velocity ($m d^{-1}$)	Condition
0.08-0.8	Typical Aquifer
21-52	5m from pumping well
215-520	1m from pumping well
1300-345	Pumping well face

Table 3: Summary of the injection strategies S1 to S22.

#Set	#Strategy	Injected Flux ($g l^{-1} s^{-1}$)			Injected Conc. ($g l^{-1}$)			Stress Periods (s)					
		F^1_{inj}	F^2_{inj}	F^3_{inj}	C^1_{inj}	C^2_{inj}	C^3_{inj}	t^1_{inj}	t^2_{inj}	t^3_{inj}	t^1_{flush}	t^2_{flush}	t^3_{flush}
Set 1	S1	62.10	-	-	16.0	-	-	525	-	-	39475	-	-
	S2	31.05	-	-	8.0	-	-	1050	-	-	37900	-	-
	S3	15.52	-	-	4.0	-	-	2100	-	-	37900	-	-
Set 2	S4	31.05	31.05	-	8.0	8.0	-	525	525	-	5700	33250	-
	S5	31.05	31.05	-	8.0	8.0	-	525	525	-	10000	28950	-
	S6	49.7	12.4	-	12.8	3.2	-	525	525	-	-	38950	-
	S7	49.7	12.4	-	12.8	3.2	-	525	525	-	5700	33250	-
	S8	49.7	12.4	-	12.8	3.2	-	525	525	-	10000	28950	-
	S9	12.4	49.7	-	3.2	12.8	-	525	525	-	-	38950	-
	S10	12.4	49.7	-	3.2	12.8	-	525	525	-	5700	33250	-
	S11	12.4	49.7	-	3.2	12.8	-	525	525	-	10000	28950	-
Set 3	S12	31.05	31.05	31.05	8.0	8.0	8.0	350	350	350	5700	5700	27550
	S13	31.05	31.05	31.05	8.0	8.0	8.0	350	350	350	10000	10000	18950
	S14	10.35	20.70	31.05	3.8	8.2	12.0	350	350	350	-	-	38950
	S15	10.35	20.70	31.05	3.8	8.2	12.0	350	350	350	5700	5700	27550
	S16	10.35	20.70	31.05	3.8	8.2	12.0	350	350	350	10000	10000	18950
	S17	31.05	20.70	10.35	12.0	8.2	3.8	350	350	350	-	-	38950
	S18	31.05	20.70	10.35	12.0	8.2	3.8	350	350	350	5700	5700	27550
	S19	31.05	20.70	10.35	12.0	8.2	3.8	350	350	350	10000	10000	18950
	S20	15.52	31.05	15.52	6.0	12.0	6.0	350	350	350	-	-	38950
	S21	15.52	31.05	15.52	6.0	12.0	6.0	350	350	350	5700	5700	27550
	S22	15.52	31.05	15.52	6.0	12.0	6.0	350	350	350	10000	10000	18950

Table 4: Summary of the injection strategies set 4 (S23 to S25).

# Set	#Strategy	F_{inj}^i ($g l^{-1} s^{-1}$)	t_{inj}^i (s)	t_{flush} (s)	C_{inj}^i ($g l^{-1}$)
Set 4	S23	3.35,5.35,7.35,9.35,11.35, 9.35, 7.35,5.35,3.35	525	35275	0.86, 1.38, 1.89, 2.41, 2.92, 2.41, 1.89, 1.38, 0.86
	S24	0.9,2.4,3.9,5.4,6.9,8.4, 9.9,11.4,12.9			0.23, 0.62, 1.0, 1.4, 1.77, 2.16, 2.55, 2.94, 3.3
	S25	12.9,11.4,9.9,8.4,6.9, 5.4,3.9,2.4,0.9			3.3, 2.94, 2.55, 2.16, 1.77, 1.4, 1.0, 0.62, 0.23